

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0-0-12	2538	HGR	1.50"	2x4 DFL #1 & Btr.
2	35-5-4	2542	HGR	1.50"	2x4 DFL #1 & Btr.
BRG HANGER/CLIP NOTE					WEB 2x4 DFL STUD
1 *HHUS26-2					Kcr (creep factor) = 2.00
2 *HHUS26-2					Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used

Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Pm = 20 psf
 End verticals are designed for axial loads only unless noted otherwise.
 Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).
 UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -1226 lb
 2 -1249 lb

2-PLY! Nail w/10d BOX, staggered (per NDS) in: TC- 2 BC- 2 WEBS- 2 **PER FOOT!** Cluster screws, if shown, are 3" long.
 Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.
 Required bearing widths and bearing areas apply when truss not supported in a hanger.
 Hip-Drop : 0-1-13
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 21.56 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Component and Cladding
 Tributary Area = 102.99 ft²

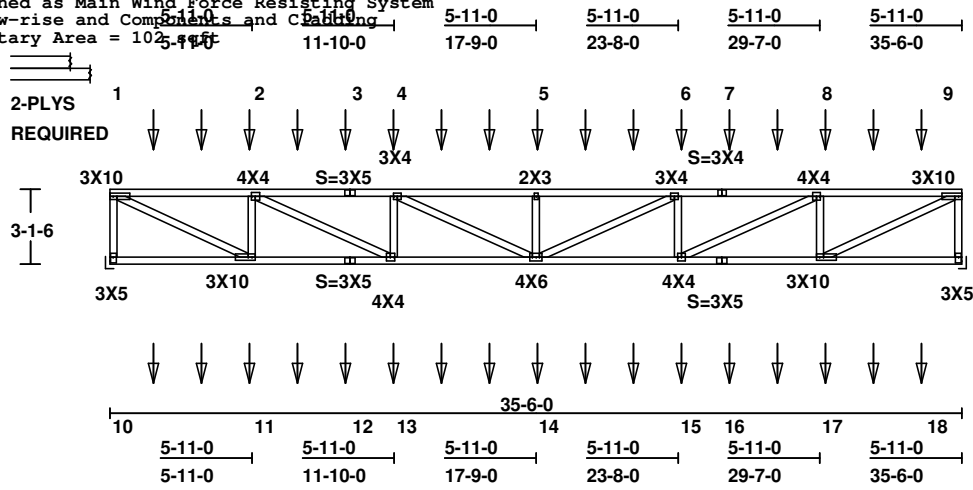
+++++
 Nail pattern shown is for PLF loads and point loads converted to PLF loads only. Concentrated loads MUST be distributed to each ply equally. Multi-ply with hangers are based on hanger nails using 1.5" nails min. into the carrying member.
 If shown, use additional fasteners for point loads as indicated from the back plys, distributed symmetrically around the hanger. Use any other approved detail (by others).
 10d = 10d NAILS, SDS = Simpson SDS screws or equivalent substitute
 (*) = Special Connection Req. (by others)
 @ = Indicates load is evenly distributed (no additional fasteners req.)
 +++++
 NAIL pattern shown is based on:
 10d BOX = 0.128" dia. x 3.0" long nail
 10d COMMON = 0.148" dia. x 3.0" long nail
 16d BOX = 0.135" dia. x 3.5" long nail
 16d COMMON = 0.162" dia. x 3.5" long nail
 +++++
 20 psf bottom chord live load NOT required on this truss, per IRC/IBC requirements for attics with limited storage.

TC	FORCE	AXL	BND	CSI
1-2	-4452	0.08	0.37	0.45
2-3	-7182	0.12	0.27	0.39
3-4	-7182	0.15	0.15	0.29
4-5	-8184	0.20	0.20	0.40
5-6	-8184	0.20	0.20	0.41
6-7	-7179	0.11	0.18	0.29
7-8	-7179	0.11	0.27	0.39
8-9	-4456	0.08	0.35	0.43

BC	FORCE	AXL	BND	CSI
10-11	0	0.00	0.13	0.13
11-12	4706	0.32	0.06	0.39
12-13	4706	0.32	0.06	0.38
13-14	7320	0.51	0.08	0.59
14-15	7316	0.50	0.08	0.59
15-16	4710	0.32	0.05	0.38
16-17	4710	0.32	0.07	0.39
17-18	0	0.00	0.13	0.13

WEB	FORCE	CSI	WEB	FORCE	CSI
1-10	-2478	0.28	6-14	967	0.16
1-11	4972	0.83	6-15	-1100	0.12
2-11	-2057	0.23	8-15	2769	0.46
2-13	2777	0.46	8-17	-2057	0.23
4-13	-1103	0.13	9-17	4976	0.83
4-14	963	0.16	9-18	-2481	0.28
5-14	-686	0.08			

LOAD CASE #1 DESIGN LOADS					
Dir	L.Plf	L.Loc	R.Plf	R.Loc	LL/TL
TC Vert	85.00	0-0-0	85.00	0-9-12	0.42
TC Vert	42.50	0-9-12	42.50	34-9-12	0.42
TC Vert	85.00	34-9-12	85.00	35-6-0	0.42
BC Vert	15.00	0-0-0	15.00	0-9-12	0.00
BC Vert	7.50	0-9-12	7.50	34-9-12	0.00
BC Vert	15.00	34-9-12	15.00	35-6-0	0.00



CRITICAL POINT LOADS				
Type	X.Loc	Max/Dur	Min/Dur	
TC Vert	1-9-12	150/1.15	-134/1.60	
TC Vert	3-9-12	150/1.15	-134/1.60	
TC Vert	5-9-12	150/1.15	-134/1.60	
TC Vert	7-9-12	150/1.15	-134/1.60	
TC Vert	9-9-12	150/1.15	-134/1.60	
TC Vert	11-9-12	150/1.15	-134/1.60	
TC Vert	13-9-12	150/1.15	-134/1.60	
TC Vert	15-9-12	150/1.15	-134/1.60	
TC Vert	17-9-12	150/1.15	-134/1.60	
TC Vert	19-9-12	150/1.15	-134/1.60	
TC Vert	21-9-12	150/1.15	-134/1.60	
TC Vert	23-9-12	150/1.15	-134/1.60	
TC Vert	25-9-12	150/1.15	-134/1.60	
TC Vert	27-9-12	150/1.15	-134/1.60	
TC Vert	29-9-12	150/1.15	-134/1.60	
TC Vert	31-9-12	150/1.15	-134/1.60	
TC Vert	33-9-12	150/1.15	-134/1.60	
BC Vert	1-9-12	64/1.25	-6/1.60	

4/15/2019

Scale: 1/8" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.00
O.C.Spacing	2-0-0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301431

BC Vert	3-	9-12	64/1.25	-6/1.60
BC Vert	5-	9-12	64/1.25	-6/1.60
BC Vert	7-	9-12	64/1.25	-6/1.60
BC Vert	9-	9-12	64/1.25	-6/1.60
BC Vert	11-	9-12	64/1.25	-6/1.60
BC Vert	13-	9-12	64/1.25	-6/1.60
BC Vert	15-	9-12	64/1.25	-6/1.60
BC Vert	17-	9-12	64/1.25	-6/1.60
BC Vert	19-	9-12	64/1.25	-6/1.60
BC Vert	21-	9-12	64/1.25	-6/1.60
BC Vert	23-	9-12	64/1.25	-6/1.60
BC Vert	25-	9-12	64/1.25	-6/1.60
BC Vert	27-	9-12	64/1.25	-6/1.60
BC Vert	29-	9-12	64/1.25	-6/1.60
BC Vert	31-	9-12	64/1.25	-6/1.60
BC Vert	33-	9-12	64/1.25	-6/1.60

MAX DEFLECTION (span) :
L/967 MEM 4-5 (LIVE) LC 1
L= -0.44" CC= -0.18" CL= -0.62"

==== Joint Locations =====

1	0-	0-	0	10	0-	0-	0
2	5-	11-	0	11	5-	11-	0
3	10-	0-	0	12	10-	0-	0
4	11-	10-	0	13	11-	10-	0
5	17-	9-	0	14	17-	9-	0
6	23-	8-	0	15	23-	8-	0
7	25-	6-	0	16	25-	6-	0
8	29-	7-	0	17	29-	7-	0
9	35-	6-	0	18	35-	6-	0

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0- 0-12	1775	HGR	1.89"	2x4 DFL #1 & Btr.
2	35- 5- 4	1775	HGR	1.89"	2x4 DFL #1 & Btr.
BRG HANGER/CLIP NOTE					
1	*HUS26				
2	*HUS26				

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Drainage must be provided to avoid ponding.
 Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.

Required bearing widths and bearing areas apply when truss not supported in a hanger.
 UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -350 lb
 2 -350 lb
 Hip-Drop : 0-1-13
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.30 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 71 sqft

Support Connection(s)/Hanger(s) are not designed for horizontal loads.
 SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC	FORCE	AXL	BND	CSI
1-2	-3296	0.08	0.52	0.60
2-3	-3017	0.05	0.25	0.30
3-4	-3688	0.16	0.38	0.54
4-5	-3713	0.10	0.39	0.49
5-6	-3713	0.10	0.37	0.47
6-7	-3713	0.16	0.35	0.52
7-8	-3017	0.05	0.26	0.31
8-9	-3297	0.09	0.42	0.52

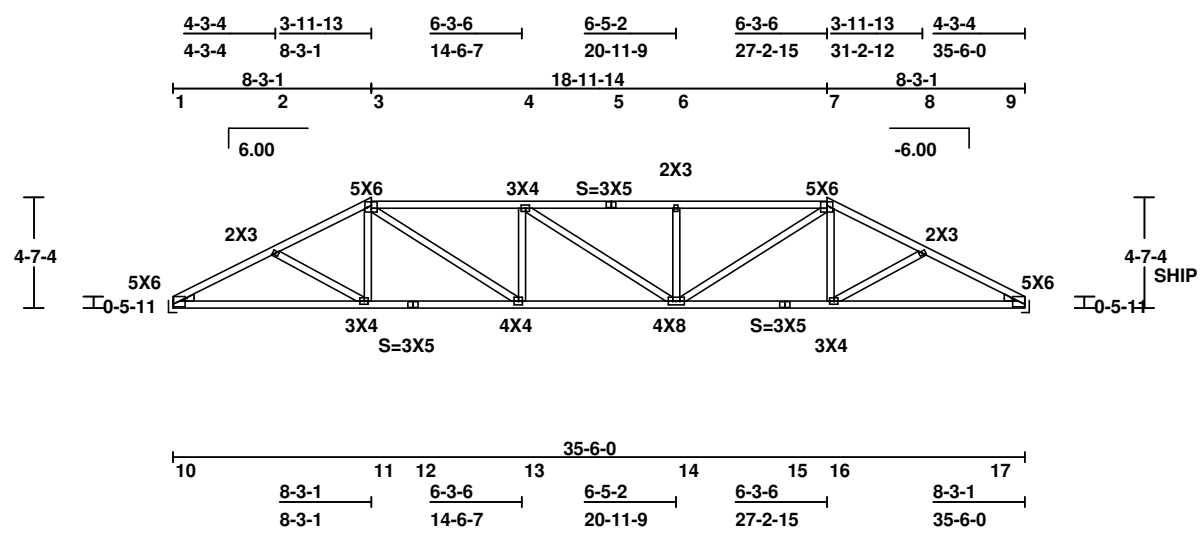
BC	FORCE	AXL	BND	CSI
10-11	2849	0.36	0.32	0.68
11-12	2639	0.33	0.08	0.41
12-13	2639	0.33	0.09	0.42
13-14	3724	0.47	0.07	0.54
14-15	2638	0.33	0.09	0.42
15-16	2638	0.33	0.08	0.41
16-17	2850	0.36	0.32	0.68

WEB	FORCE	CSI	WEB	FORCE	CSI
2-11	-492	0.19	6-14	-553	0.19
3-11	336	0.11	7-14	1352	0.45
3-13	1326	0.44	7-16	338	0.11
4-13	-605	0.21	8-16	-493	0.19
4-14	-162	0.17			

MAX DEFLECTION (span) :
 L/999 MEM 3-4 (LIVE) LC 1
 L= -0.37" CC= -0.13" CL= -0.50"

==== Joint Locations =====

1	0- 0- 0	10	0- 0- 0
2	4- 3- 4	11	8- 3- 1
3	8- 3- 1	12	10- 0- 0
4	14- 6- 7	13	14- 6- 7
5	18- 3- 1	14	20-11- 9
6	20-11- 9	15	25- 6- 0
7	27- 2-15	16	27- 2-15
8	31- 2-12	17	35- 6- 0
9	35- 6- 0		



4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 1/8" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.10
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301432

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 0-12	1775	HGR	1.89"
2	35- 5- 4	1775	HGR	1.89"

TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 WEB 2x4 DFL STUD
 PLT BLK 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Pm = 20 psf
 Lu Parallel to Ridge = 20-0-0
 Minimum Lu = 20-00-00
 20 psf bottom chord live load NOT required on this truss, per IRC/IBC requirements for attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Drainage must be provided to avoid ponding.
 Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.

Required bearing widths and bearing areas apply when truss not supported in a hanger.
 UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -355 lb
 2 -355 lb
 Hip-Drop : 0-1-11
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.97 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 71 sqft

Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC	FORCE	AXL	BND	CSI
1-2	-3257	0.08	0.44	0.51
2-3	-2810	0.04	0.36	0.40
3-4	-2924	0.04	0.56	0.60
4-5	-2924	0.04	0.56	0.60
5-6	-2810	0.04	0.36	0.40
6-7	-3257	0.08	0.44	0.51

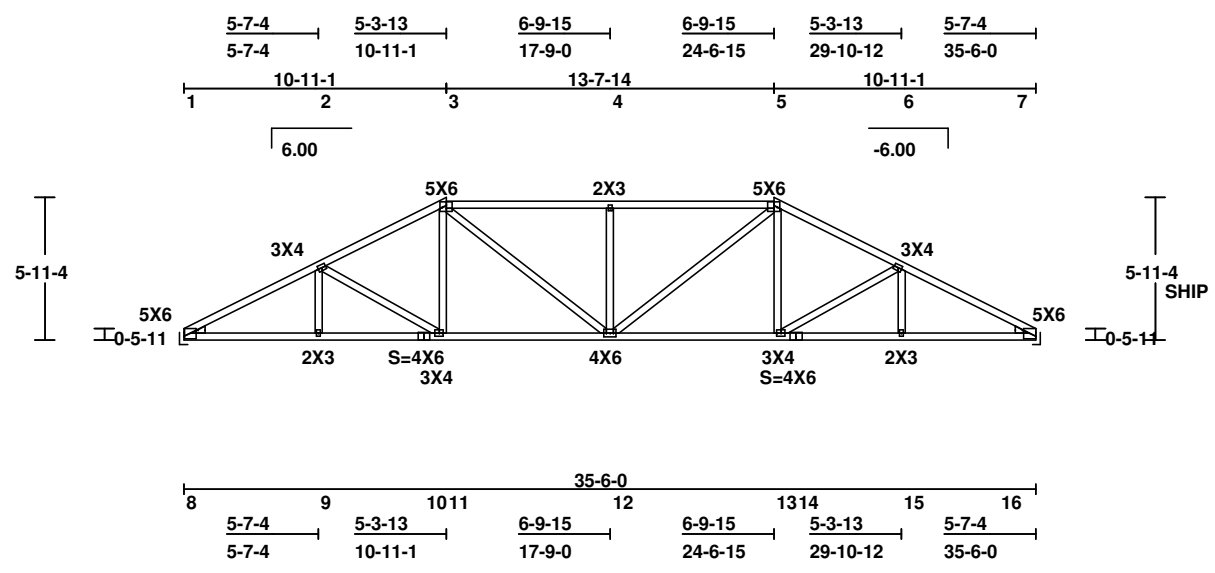
BC	FORCE	AXL	BND	CSI
8-9	2807	0.35	0.31	0.66
9-10	2807	0.35	0.10	0.46
10-11	2807	0.35	0.11	0.46
11-12	2425	0.30	0.11	0.41
12-13	2425	0.30	0.11	0.41
13-14	2807	0.35	0.11	0.46
14-15	2807	0.35	0.10	0.46
15-16	2807	0.35	0.31	0.66

WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	141	0.04	5-12	769	0.26
2-11	-747	0.47	5-13	496	0.17
3-11	496	0.17	6-13	-747	0.47
3-12	769	0.26	6-15	141	0.04
4-12	-667	0.37			

MAX DEFLECTION (span) :
 L/999 MEM 4-5 (LIVE) LC 1
 L= -0.33" CC= -0.12" CL= -0.45"

==== Joint Locations =====

1	0- 0- 0	9	5- 7- 4
2	5- 7- 4	10	10- 0- 0
3	10-11- 1	11	10-11- 1
4	17- 9- 0	12	17- 9- 0
5	24- 6-15	13	24- 6-15
6	29-10-12	14	25- 6- 0
7	35- 6- 0	15	29-10-12
8	0- 0- 0	16	35- 6- 0



4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 1/8" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.10
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301433

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0-0-12	1905 HGR	2.03"		2x4 DFL #1 & Btr.
2	35-5-4	1905 HGR	2.03"		2x4 DFL #1 & Btr.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 Kcr (creep factor) = 2.00
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Pm = 20 psf
 Lu Parallel to Ridge = 20-0-0
 Minimum Lu = 20-00-00
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

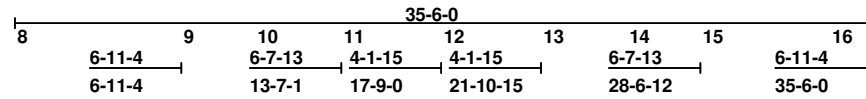
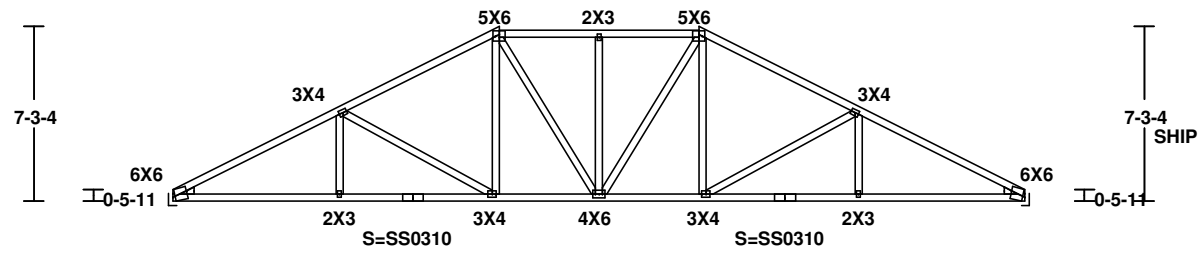
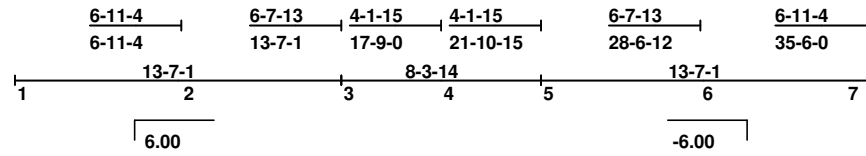
Required bearing widths and bearing areas apply when truss not supported in a hanger.
 UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -360 lb
 2 -360 lb
 Hip-Drop : 0-1-13
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 23.63 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 71 sqft

Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC	FORCE	AXL	BND	CSI
1-2	-3500	0.06	0.58	0.64
2-3	-2673	0.04	0.60	0.64
3-4	-2308	0.04	0.28	0.32
4-5	-2308	0.04	0.28	0.32
5-6	-2673	0.04	0.60	0.64
6-7	-3500	0.06	0.58	0.64

BC	FORCE	AXL	BND	CSI
8-9	3008	0.38	0.26	0.64
9-10	3007	0.38	0.10	0.47
10-11	3007	0.38	0.09	0.47
11-12	2219	0.28	0.09	0.37
12-13	2219	0.28	0.09	0.37
13-14	3007	0.38	0.09	0.47
14-15	3007	0.38	0.10	0.47
15-16	3008	0.38	0.26	0.64

WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	223	0.07	5-12	410	0.20
2-11	-957	0.93	5-13	581	0.19
3-11	581	0.19	6-13	-957	0.93
3-12	410	0.20	6-15	223	0.07
4-12	-397	0.33			



MAX DEFLECTION (span) :
 L/999 MEM 2-3 (LIVE) LC 47
 L= -0.36" CC= -0.10" CL= -0.46"

==== Joint Locations =====

1	0-0-0	9	6-11-4
2	6-11-4	10	10-0-0
3	13-7-1	11	13-7-1
4	17-9-0	12	17-9-0
5	21-10-15	13	21-10-15
6	28-6-12	14	25-6-0
7	35-6-0	15	28-6-12
8	0-0-0	16	35-6-0

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 1/8" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.10
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301434

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0- 2-12	2144	5.50"	2.29"	BC
2	35- 9- 4	2341	5.50"	2.50"	

TC	FORCE	AXL	BND	CSI
1-2	-7316	0.56	0.36	0.91
2-3	-6981	0.48	0.35	0.84
3-4	-3231	0.10	0.34	0.44
4-5	-2758	0.07	0.13	0.20
5-6	-2969	0.06	0.43	0.50
6-7	-4053	0.11	0.43	0.54
7-8	-4349	0.14	0.36	0.50

BC	FORCE	AXL	BND	CSI
9-10	6667	0.49	0.29	0.78
10-11	6740	0.50	0.29	0.79
11-12	5185	0.65	0.16	0.81
12-13	2577	0.32	0.10	0.43
13-14	3193	0.40	0.30	0.70
14-15	3792	0.48	0.37	0.85

2x4 DFL #1 & Btr.
 2x4 DFL #1 & Btr.
 2x6 DFL SS 9-11
 2x4 DFL STUD
 2x4 DFL #1 & Btr. 11-3
 2x4 DFL #1 & Btr.
 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 + + + + +
 Unrestrained horiz. LL deflection = 0.45"
 + + + + +
 Drainage must be provided to avoid ponding.

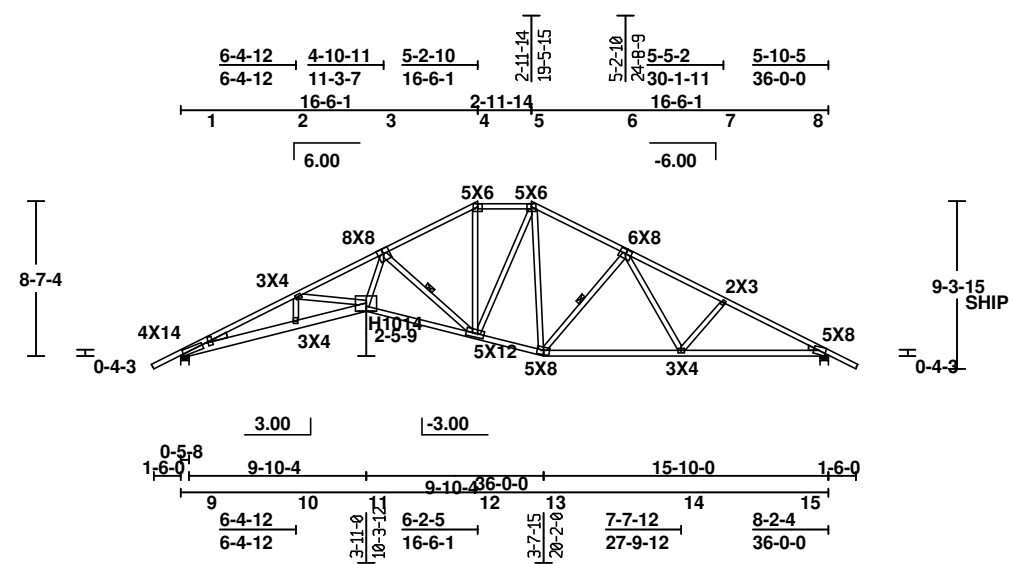
Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details.
 [PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Pm = 20 psf
 Lu Parallel to Ridge = 20-0-0
 Minimum Lu = 20-00-00
 20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -434 lb
 2 -434 lb
 Hip-Drop : 0-1-11
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 24.30 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 72 sqft

MAX DEFLECTION (span) :
 L/542 MEM 2-3 (LIVE) LC 47
 L= -0.79" CC= -0.25" CL= -1.03"

==== Joint Locations =====

1	0- 0- 0	9	0- 0- 0
2	6- 4-12	10	6- 4-12
3	11- 3- 7	11	10- 3-12
4	16- 6- 1	12	16- 6- 1
5	19- 5-15	13	20- 2- 0
6	24- 8- 9	14	27- 9-12
7	30- 1-11	15	36- 0- 0
8	36- 0- 0		



TYPICAL PLATE : 3X6

4/15/2019

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Scale: 3/32" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.10
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301435

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0- 2-12	2098	5.50"	2.24"	BC
2	35- 9- 4	2297	5.50"	2.45"	

TC	FORCE	AXL	BND	CSI
1-2	-6223	0.47	0.52	0.98
2-3	-5921	0.43	0.44	0.87
3-4	-3012	0.06	0.40	0.46
4-5	-2543	0.04	0.43	0.48
5-6	-3559	0.08	0.42	0.50
6-7	-3760	0.17	0.33	0.50

BC	FORCE	AXL	BND	CSI
8-9	5617	0.41	0.30	0.72
9-10	5826	0.43	0.30	0.73
10-11	3557	0.45	0.17	0.62
11-12	2126	0.24	0.14	0.39
12-13	2561	0.32	0.31	0.64
13-14	3253	0.41	0.31	0.72

WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	-275	0.06	4-12	710	0.24
2-10	-451	0.13	5-12	-1068	0.97
3-10	3101	0.43	5-13	904	0.30
3-11	-2124	0.79	6-13	-463	0.14
4-11	1729	0.58			

2x4 DFL #1 & Btr.
 2x4 DFL #1 & Btr.
 2x6 DFL SS 8-10
 2x4 DFL STUD
 2x4 DFL #1 & Btr. 10-3
 2x4 DFL #1 & Btr.
 PLT BLK
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 20 psf Live Load applied to bottom chord in accordance with IRC/IBC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

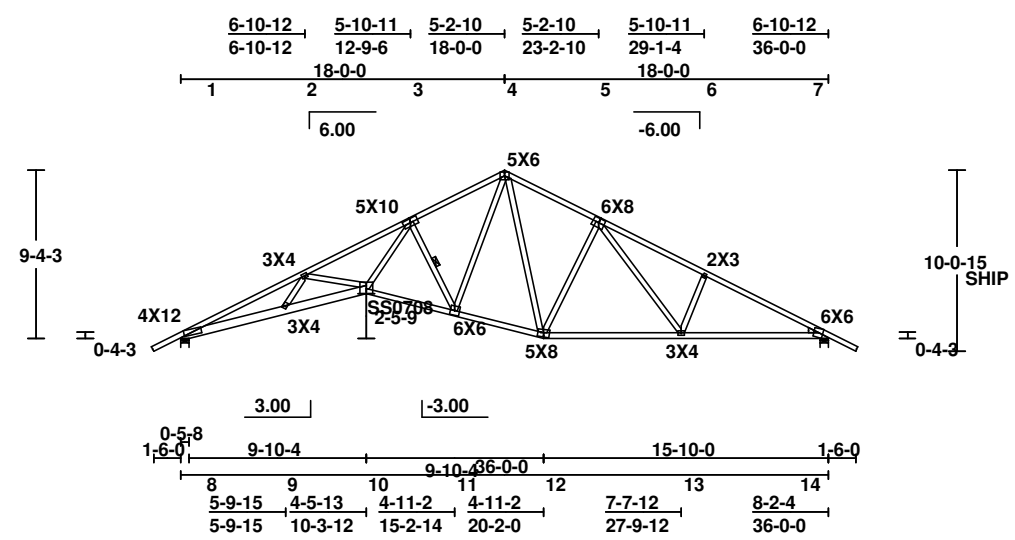
Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details.
 [PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 + + + + + Unrestrained horiz. LL deflection = 0.27 "

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -420 lb
 2 -420 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 24.67 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 72 sqft

MAX DEFLECTION (span) :
 L/804 MEM 2-3 (LIVE) LC 43
 L= -0.53" CC= -0.20" CL= -0.73"

==== Joint Locations =====

1	0- 0- 0	8	0- 0- 0
2	6-10-12	9	5- 9-15
3	12- 9- 6	10	10- 3-12
4	18- 0- 0	11	15- 2-14
5	23- 2-10	12	20- 2- 0
6	29- 1- 5	13	27- 9-12
7	36- 0- 0	14	36- 0- 0



4/15/2019

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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.15
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301436

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0- 2-12	1923	5.50"	2.05"	BC
2	32- 9- 4	1892	5.50"	2.02"	

TC	FORCE	AXL	BND	CSI
1-2	-5634	0.39	0.48	0.87
2-3	-5264	0.32	0.37	0.69
3-4	-2591	0.05	0.40	0.45
4-5	-2040	0.03	0.43	0.46
5-6	-2289	0.03	0.39	0.42
6-7	102	0.01	0.23	0.25

BC	FORCE	AXL	BND	CSI
8-9	5080	0.37	0.26	0.64
9-10	5250	0.39	0.26	0.65
10-11	3132	0.39	0.17	0.56
11-12	1748	0.21	0.11	0.31
12-13	1931	0.24	0.26	0.50
13-14	1876	0.24	0.26	0.50

2x4 DFL #1 & Btr.
 2x4 DFL #1 & Btr.
 2x6 DFL SS 8-10
 2x4 DFL STUD

WEB
 2x4 DFL STUD

Designed per ANSI/TPI 1-2014
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00

HORIZONTAL REACTION(S):
 support 1 117 lb
 support 2 117 lb

20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details.
 [PM]=PLATE MONITOR USED-See Joint Report
 Fabrication Tolerance = 20.0%
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 24.67 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 66 sqft

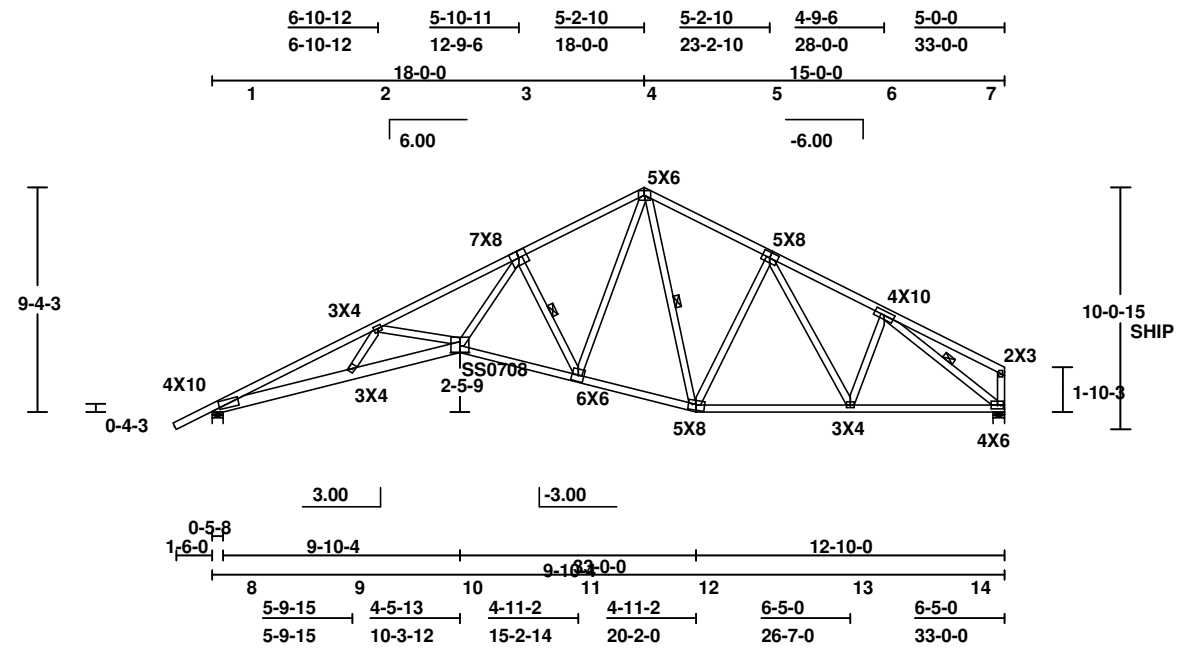
End verticals are designed for axial loads only unless noted otherwise.
 Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).
 UPLIFT REACTION(S):
 Support C&C Wind Non-Wind
 1 -395 lb
 2 -326 lb

WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	-220	0.05	5-12	-660	0.60
2-10	-497	0.15	5-13	82	0.04
3-10	2798	0.94	6-13	330	0.11
3-11	-1995	0.69	6-14	-2481	0.76
4-11	1636	0.55	7-14	-179	0.03
4-12	387	0.16			

MAX DEFLECTION (span):
 L/888 MEM 2-3 (LIVE) LC 43
 L= -0.44" CC= -0.17" CL= -0.61"

==== Joint Locations =====

1	0- 0- 0	8	0- 0- 0
2	6-10-12	9	5- 9-15
3	12- 9- 6	10	10- 3-12
4	18- 0- 0	11	15- 2-14
5	23- 2-10	12	20- 2- 0
6	28- 0- 0	13	26- 7- 0
7	33- 0- 0	14	33- 0- 0



4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 1/8" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301437

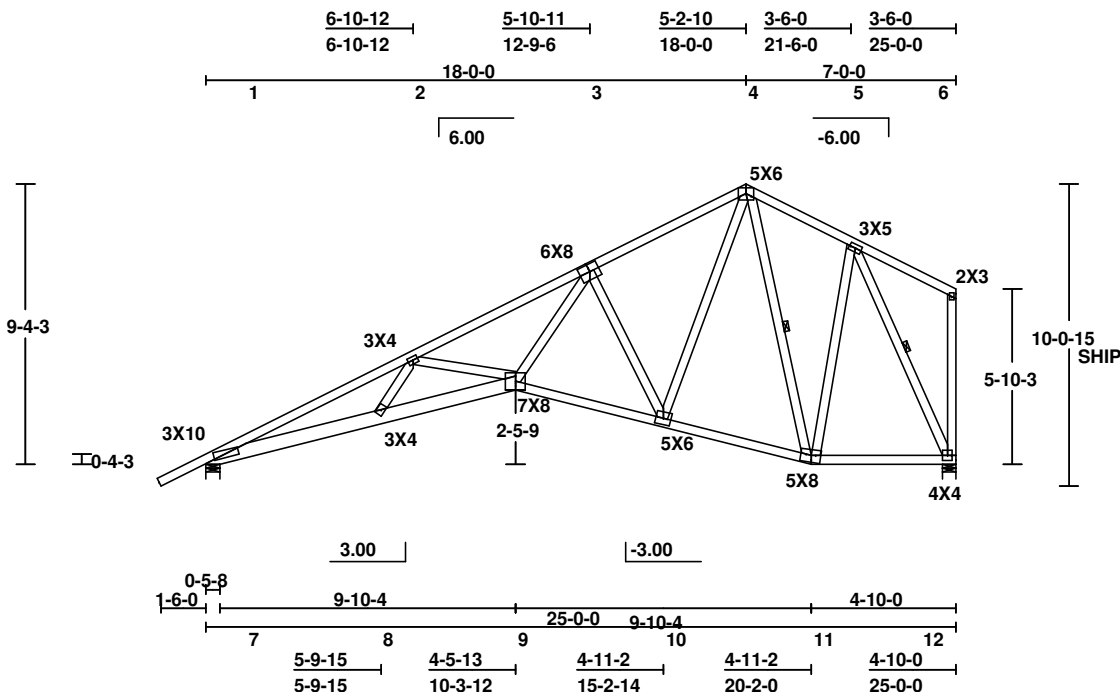
BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0- 2-12	1460	5.50"	1.56"	2x4 DFL #1 & Btr.
2	24- 9- 4	1250	5.50"	1.50"	2x4 DFL #1 & Btr.
					2x6 DFL SS 7-9
					2x4 DFL STUD
TC	FORCE	AXL	BND	CSI	
1-2	-4145	0.20	0.37	0.57	
2-3	-3585	0.08	0.40	0.48	
3-4	-1565	0.02	0.41	0.42	
4-5	-714	0.00	0.22	0.22	
5-6	120	0.01	0.22	0.24	
BC	FORCE	AXL	BND	CSI	
7-8	3723	0.27	0.17	0.45	
8-9	3808	0.28	0.17	0.45	
9-10	2021	0.25	0.14	0.39	
10-11	814	0.04	0.14	0.18	
11-12	478	0.03	0.11	0.14	
WEB	FORCE	CSI	WEB	FORCE	CSI
2-8	125	0.04	4-11	-787	0.33
2-9	-613	0.18	5-11	651	0.22
3-9	2045	0.68	5-12	-1191	0.63
3-10	-1631	0.98	6-12	-171	0.10
4-10	1355	0.45			

Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details. **[PM]=PLATE MONITOR USED-See Joint Report** Fabrication Tolerance = 20.0% Kcr (creep factor) = 2.00 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118. This truss is designed using the ASCE7-10 Wind Specification Bldg Enclosed = Yes, Truss Location = Not End Zone Exp Category = C Bldg Length = 99.00 ft, Bldg Width = 50.00 ft Mean roof height = 24.67 ft, mph = 115 Occupancy Category II, Dead Load = 9.0 psf Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding Tributary Area = 50 sqft

End verticals are designed for axial loads only unless noted otherwise. Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others). UPLIFT REACTION(S) : Support C&C Wind Non-Wind
1 -309 lb
2 -295 lb

DESIGNED PER ANSI/TPI 1-2014
Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
ASCE7-10 SNOW LOAD DESIGN CRITERIA:
Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
HORIZONTAL REACTION(S) :
support 1 203 lb
support 2 203 lb
20 psf bottom chord live load NOT required on this truss, per IRC/IBC requirements for attics with limited storage.

MAX DEFLECTION (span) :
L/999 MEM 2-3 (LIVE) LC 41
L= -0.27" CC= -0.10" CL= -0.37"



4/15/2019

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Scale: 5/32" = 1'



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Eng. Job: .EJ.
Chk: BJS
Dsgnr: BJS

TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO: FC BELMONT 2-1-30
DurFacs L=1.15 P=1.15
Rep Mbr Bnd 1.15
O.C.Spacing 2- 0- 0
Design Spec IRC-2015
Seqn T6.5.21 - 301438

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0- 2-12	1750	5.50"	1.87"	2x4 DFL #1 & Btr.
2	24- 9- 4	1586	5.50"	1.69"	2x4 DFL #1 & Btr.

TC	FORCE	AXL	BND	CSI
1-2	-2804	0.04	0.35	0.39
2-3	-2440	0.04	0.46	0.50
3-4	-1236	0.01	0.46	0.47
4-5	-1068	0.01	0.20	0.21
5-6	119	0.01	0.20	0.21

BC	FORCE	AXL	BND	CSI
7-8	2417	0.30	0.38	0.68
8-9	1689	0.21	0.39	0.60
9-10	1689	0.21	0.42	0.63
10-11	594	0.07	0.39	0.47

WEB	FORCE	CSI	WEB	FORCE	CSI
2-8	-512	0.19	5-10	830	0.28
3-8	829	0.28	5-11	-1463	0.63
3-10	-1170	0.42	6-11	-182	0.10
4-10	537	0.18			

Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details.

Designed per ANSI/TPI 1-2014

Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.

Loaded for 10 PSF non-concurrent BCLL.

ASCE7-10 SNOW LOAD DESIGN CRITERIA:
Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00

20 psf Live load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

[PM]=PLATE MONITOR USED-See Joint Report

Fabrication Tolerance = 20.0%

Kcr (creep factor) = 2.00

Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used

IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.

End verticals are designed for axial loads only unless noted otherwise.

Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).

UPLIFT REACTION(S) :

Support	C&C Wind	Non-Wind
1	-309 lb	
2	-295 lb	

HORIZONTAL REACTION(S) :

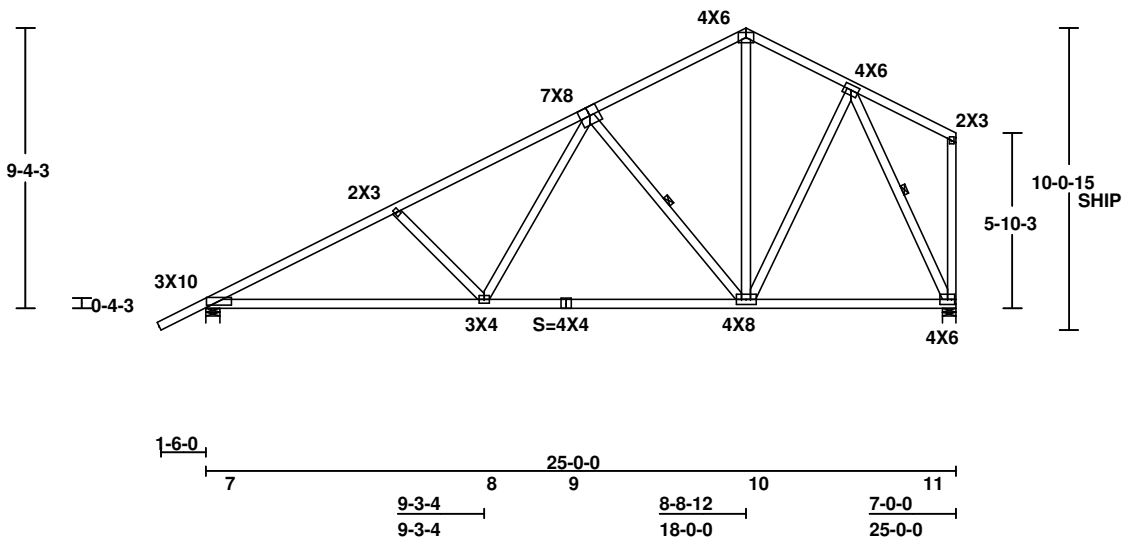
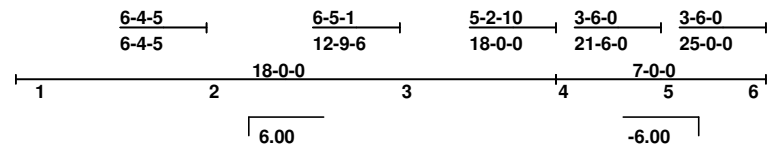
support	1	202 lb
support	2	202 lb

This truss is designed using the ASCE7-10 Wind Specification

Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = C
Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
Mean roof height = 24.67 ft, mph = 115
Occupancy Category II, Dead Load = 9.0 psf
Designed as Main Wind Force Resisting System
- Low-rise and Components and Cladding
Tributary Area = 50 sqft

MAX DEFLECTION (span) :

L/999 MEM 9-10 (LIVE) LC 41
L= -0.22" CC= -0.07" CL= -0.28"



==== Joint Locations =====

1	0- 0- 0	7	0- 0- 0
2	6- 4- 5	8	9- 3- 4
3	12- 9- 6	9	12- 0- 0
4	18- 0- 0	10	18- 0- 0
5	21- 6- 0	11	25- 0- 0
6	25- 0- 0		

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 5/32" = 1'



P.O.BOX 787, BLACKFOOT, ID 83221

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.

This design is for an individual building component not truss system. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer must ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code and the particular application. The design assumes that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood to exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: 'Joint and Cutting Detail Reports' available as output from Truswal software, 'ANSI/TPI 1', 'WTCA 1' - Wood Truss Council of America Standard Design Responsibilities, 'BUILDING COMPONENT SAFETY INFORMATION' - (BCSI) and 'BCSI SUMMARY SHEETS' by WTCA and TPI. The Truss Plate Institute (TPI) is located at 218 N. Lee Street Suite 312, Alexandria, VA 22314. The American Forest and Paper Association (AFPA) is located at 1111 19th Street, NW, Ste 800, Washington, DC 20036.

Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301439

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-0-12	2406	HGR	1.50"
2	24-6-4	2604	5.50"	1.50"

BRG HANGER/CLIP NOTE
1 *HHUS26-2

Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC 2x4 DFL #1 & Btr.
BC 2x4 DFL #1 & Btr.
WEB 2x4 DFL STUD
Kcr (creep factor) = 2.00

Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118. Drainage must be provided to avoid ponding. Permanent bracing is required (by others) to prevent rotation/toppling. See BCSI and ANSI/TPI 1.

2-PLY! Nail w/10d BOX, staggered (per NDS) in: TC- 2 BC- 2 WEBS- 2 **PER FOOT!** Cluster screws, if shown, are 3" long. Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.

Required bearing widths and bearing areas apply when truss not supported in a hanger.

UPLIFT REACTION(S) :
Support C&C Wind Non-Wind
1 -1078 lb
2 -1398 lb

[PM]=PLATE MONITOR USED-See Joint Report
Designed per ANSI/TPI 1-2014
Fabrication Tolerance = 20.0%
Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
ASCE7-10 SNOW LOAD DESIGN CRITERIA:
Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
Pm = 20 psf

End verticals are designed for axial loads only unless noted otherwise.
Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).

HORIZONTAL REACTION(S) :
support 1 146 lb
support 2 103 lb

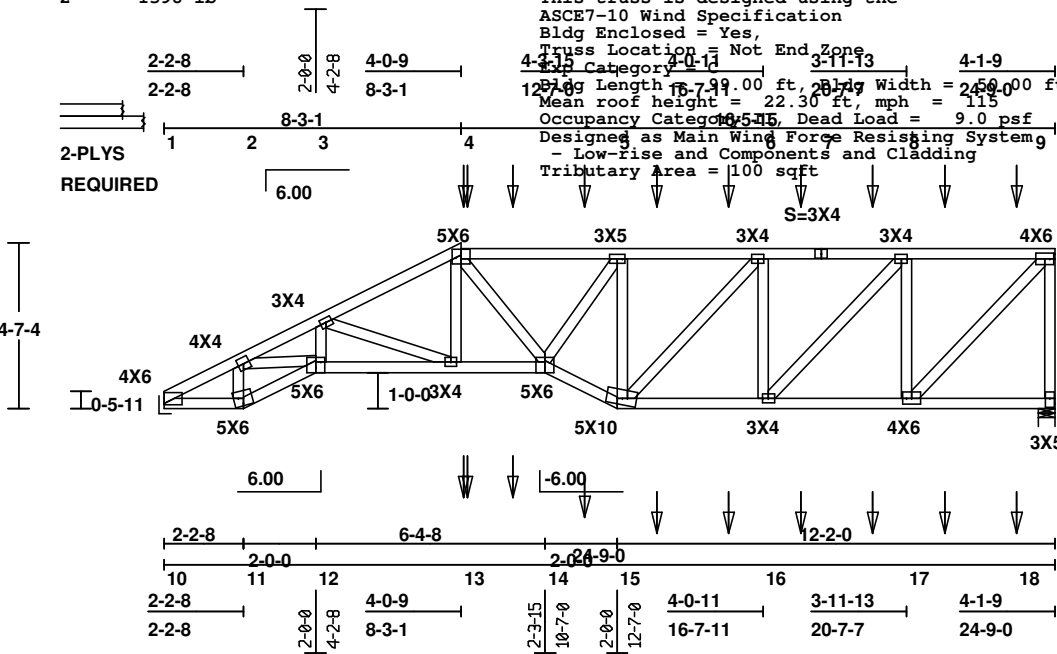
This truss is designed using the ASCE7-10 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Risk Category = II
Mean roof height = 22.30 ft, Avg. Width = 259.00 ft
Occupancy Category = U5
Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding
Tributary Area = 100 sqft

+++++
Nail pattern shown is for PLF loads and point loads converted to PLF loads only. Concentrated loads MUST be distributed to each ply equally. Multi-ply with hangers are based on hanger nails using 1.5" nails min. into the carrying member.
If shown, use additional fasteners for point loads as indicated from the back plys, distributed symmetrically around the hanger. Use any other approved detail (by others).
10d = 10d NAILS, SDS = Simpson SDS screws or equivalent substitute
(*) = Special Connection Req. (by others)
(@ = Indicates load is evenly distributed (no additional fasteners req.)
+++++
NAIL pattern shown is based on:
10d BOX = 0.128" dia. x 3.0" long nail
10d COMMON = 0.148" dia. x 3.0" long nail
16d BOX = 0.135" dia. x 3.5" long nail
16d COMMON = 0.162" dia. x 3.5" long nail
+++++
Hip-Drop : 0-1-13
20 psf bottom chord live load NOT required on this truss, per IRC requirements for attics with limited storage.

TC	FORCE	AXL	BND	CSI
1-2	-4220	0.04	0.23	0.27
2-3	-7743	0.15	0.16	0.31
3-4	-5689	0.08	0.13	0.21
4-5	-5309	0.08	0.17	0.25
5-6	-4104	0.11	0.10	0.21
6-7	-3432	0.09	0.09	0.18
7-8	-3432	0.09	0.13	0.22
8-9	-2067	0.01	0.19	0.20

BC	FORCE	AXL	BND	CSI
10-11	3637	0.25	0.23	0.48
11-12	4142	0.28	0.16	0.45
12-13	6624	0.46	0.13	0.58
13-14	4973	0.34	0.21	0.55
14-15	4594	0.31	0.05	0.36
15-16	3492	0.24	0.04	0.28
16-17	2169	0.15	0.04	0.19
17-18	103	0.00	0.08	0.08

WEB	FORCE	CSI	WEB	FORCE	CSI
2-11	-1982	0.19	5-15	-2508	0.43
2-12	3213	0.54	6-15	891	0.15
3-12	1396	0.23	6-16	-1238	0.21
3-13	-1863	0.29	8-16	1878	0.31
4-13	1593	0.27	8-17	-2099	0.36
4-14	558	0.09	9-17	3066	0.51
5-14	2249	0.38	9-18	-2536	0.43



-----LOAD CASE #1 DESIGN LOADS-----

Dir	L.Plf	L.Loc	R.Plf	R.Loc	LL/TL
TC Vert	85.00	0-0-0	85.00	7-3-13	0.42
TC Vert	42.50	7-3-13	42.50	24-8-4	0.42
BC Vert	15.00	0-0-0	15.00	7-3-13	0.00
BC Vert	7.50	7-3-13	7.50	24-8-4	0.00

-----CRITICAL POINT LOADS-----

Type	X.Loc	Max/Dur	Min/Dur
TC Vert	8-3-13	161/1.15	-125/1.60
TC Vert	8-5-1	45/1.15	-10/1.60
TC Vert	9-8-4	161/1.15	-125/1.60
TC Vert	11-8-4	161/1.15	-125/1.60
TC Vert	13-8-4	227/1.15	-194/1.60
TC Vert	15-8-4	227/1.15	-194/1.60
TC Vert	17-8-4	227/1.15	-194/1.60
TC Vert	19-8-4	227/1.15	-194/1.60
TG Vert	4-7-4	227/1.15	-194/1.60
TC Vert	23-8-4	227/1.15	-194/1.60
BC Vert	8-3-13	129/1.15	-75/1.60
BC Vert	8-5-1	722/1.15	-367/1.60
BC Vert	9-8-4	129/1.15	-75/1.60
BC Vert	11-8-4	129/1.15	-75/1.60
BC Vert	13-8-4	97/1.25	-5/1.60
BC Vert	15-8-4	97/1.25	-5/1.60
BC Vert	17-8-4	97/1.25	-5/1.60
BC Vert	19-8-4	97/1.25	-5/1.60
BC Vert	21-8-4	97/1.25	-5/1.60

4/15/2019

Scale: 3/16" = 1'



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Eng. Job: .EJ.
Chk: BJS
Dsgnr: BJS

TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO: FC BELMONT 2-1-30

DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.00
O.C.Spacing	2-0-0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301440

MAX DEFLECTION (span) :
L/999 MEM 4-5 (LIVE) LC 1
L= -0.18" CC= -0.07" CL= -0.25"

==== Joint Locations ====

1	0- 0- 0	10	0- 0- 0
2	2- 2- 8	11	2- 2- 8
3	4- 2- 8	12	4- 2- 8
4	8- 3- 1	13	8- 3- 1
5	12- 7- 0	14	10- 7- 0
6	16- 7-11	15	12- 7- 0
7	18- 3- 1	16	16- 7-11
8	20- 7- 7	17	20- 7- 7
9	24- 9- 0	18	24- 9- 0

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

BRG X-LOC REACT SIZE REQ'D TC
1 0- 0-12 1292 HGR 1.50"
2 24- 6- 4 1245 5.50" 1.50"
BRG HANGER/CLIP NOTE
1 *HUS26
Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC 2x4 DFL #1 & Btr.
BC 2x4 DFL #1 & Btr.
WEB 2x4 DFL STUD
Kcr (creep factor) = 2.00
Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118. Drainage must be provided to avoid ponding. End verticals are designed for axial loads only unless noted otherwise. Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).

[PM]=PLATE MONITOR USED-See Joint Report
Designed per ANSI/TPI 1-2014
Fabrication Tolerance = 20.0%
Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
ASCE7-10 SNOW LOAD DESIGN CRITERIA:
Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
Pm = 20 psf
Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.
20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

Required bearing widths and bearing areas apply when truss not supported in a hanger.
UPLIFT REACTION(S) :
Support C&C Wind Non-Wind
1 -231 lb
2 -272 lb
HORIZONTAL REACTION(S) :
support 1 205 lb
support 2 205 lb
Hip-Drop : 0-1-13
This truss is designed using the ASCE7-10 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = C
Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
Mean roof height = 22.97 ft, mph = 115
Occupancy Category II, Dead Load = 9.0 psf
Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding
Tributary Area = 50 sqft

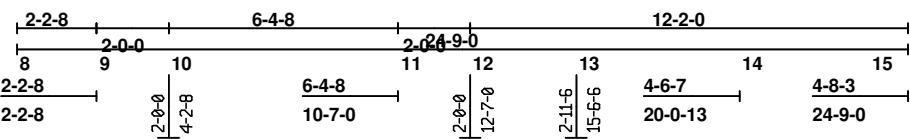
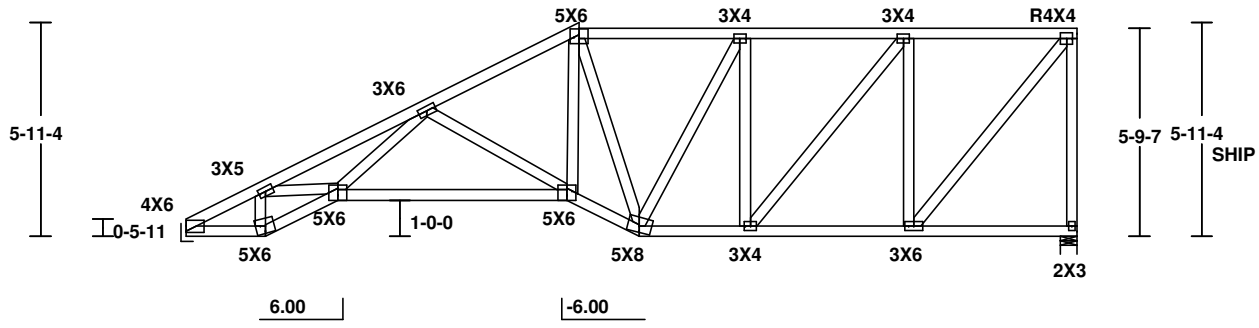
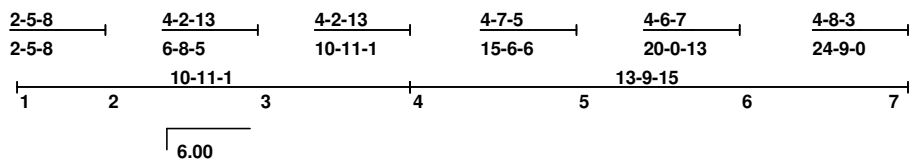
Table with columns: TC, FORCE, AXL, BND, CSI. Rows 1-2 to 6-7.

Table with columns: BC, FORCE, AXL, BND, CSI. Rows 8-9 to 13-14.

Table with columns: WEB, FORCE, CSI. Rows 2-9 to 4-12.

MAX DEFLECTION (span) :
L/999 MEM 3-4 (LIVE) LC 41
L= -0.16" CC= -0.06" CL= -0.22"

Table with columns: Joint Locations. Rows 1-8.



4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted. Scale: 3/16" = 1'



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Table with columns: Eng. Job, Chk, Dsgnr, TC Live, TC Dead, BC Live, BC Dead, TOTAL, DurFacs, Rep Mbr Bnd, O.C.Spacing, Design Spec, Seqn.

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-0-12	1403	HGR	1.50"
2	24-6-4	1504	5.50"	1.60"

BRG HANGER/CLIP NOTE
1 *HUS26

Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC 2x4 DFL #1 & Btr.
BC 2x4 DFL #1 & Btr.
WEB 2x4 DFL STUD
[PM]=PLATE MONITOR USED-See Joint Report
Fabrication Tolerance = 20.0%
Kcr (creep factor) = 2.00
Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
Drainage must be provided to avoid ponding.
Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.
Hip-Drop : 0-1-13
20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

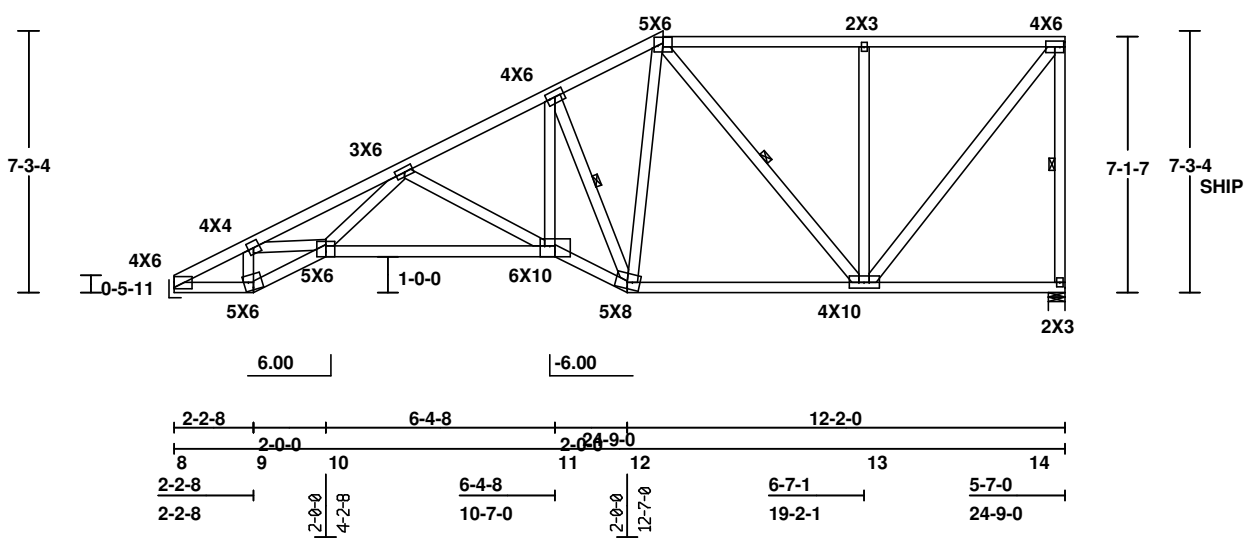
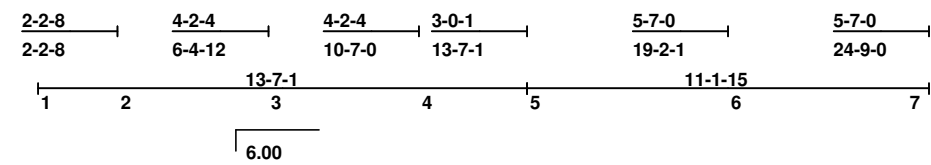
Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details.
Designed per ANSI/TPI 1-2014
Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
ASCE7-10 SNOW LOAD DESIGN CRITERIA:
Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
Pm = 20 psf
HORIZONTAL REACTION(S) :
support 1 243 lb
support 2 243 lb

End verticals are designed for axial loads only unless noted otherwise.
Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).
Required bearing widths and bearing areas apply when truss not supported in a hanger.
UPLIFT REACTION(S) :
Support C&C Wind Non-Wind
1 -216 lb
2 -295 lb
This truss is designed using the ASCE7-10 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = C
Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
Mean roof height = 23.63 ft, mph = 115
Occupancy Category II, Dead Load = 9.0 psf
Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding
Tributary Area = 50 sqft

TC	FORCE	AXL	BND	CSI
1-2	-2441	0.04	0.23	0.28
2-3	-4320	0.18	0.28	0.46
3-4	-2263	0.04	0.23	0.27
4-5	-1550	0.01	0.20	0.21
5-6	-945	0.01	0.40	0.40
6-7	-945	0.01	0.40	0.40

BC	FORCE	AXL	BND	CSI
8-9	2109	0.26	0.31	0.58
9-10	2427	0.30	0.09	0.40
10-11	2949	0.37	0.14	0.51
11-12	2162	0.27	0.23	0.50
12-13	1264	0.15	0.27	0.42
13-14	244	0.00	0.21	0.21

WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	-1192	0.23	5-12	876	0.29
2-10	1664	0.56	5-13	-658	0.26
3-10	1271	0.43	6-13	-570	0.48
3-11	-1165	0.49	7-13	1531	0.51
4-11	1509	0.50	7-14	-1396	0.95
4-12	-1737	0.90			



MAX DEFLECTION (span) :
L/999 MEM 3-4 (LIVE) LC 41
L = -0.21" CC = -0.07" CL = -0.28"

==== Joint Locations ====

1	0-0-0	8	0-0-0
2	2-2-8	9	2-2-8
3	6-4-12	10	4-2-8
4	10-7-0	11	10-7-0
5	13-7-1	12	12-7-0
6	19-2-1	13	19-2-1
7	24-9-0	14	24-9-0

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 3/16" = 1'



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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.10
BC Live 0.00 psf	O.C.Spacing 2-0-0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301442

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-0-12	1409	HGR	1.50"
2	24-6-4	1498	5.50"	1.60"

BRG HANGER/CLIP NOTE
1 *HUS26

Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC 2x4 DFL #1 & Btr.
BC 2x4 DFL #1 & Btr.
WEB 2x4 DFL STUD
[PM]=PLATE MONITOR USED-See Joint Report
Fabrication Tolerance = 20.0%
Kcr (creep factor) = 2.00
Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
Drainage must be provided to avoid ponding. Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.
Hip-Drop : 0-1-13
20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

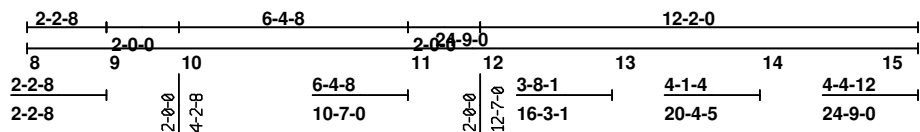
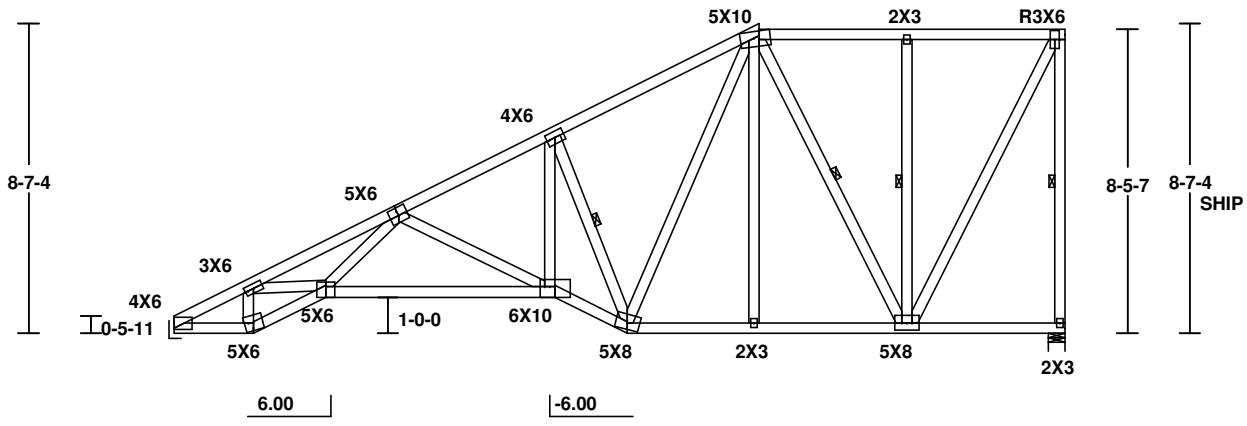
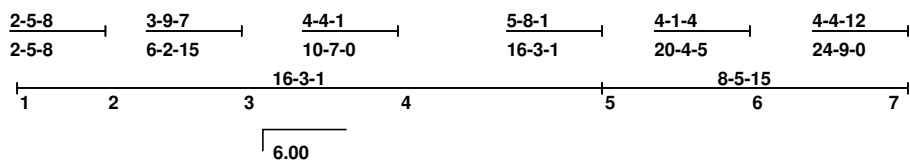
Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details.
Designed per ANSI/TPI 1-2014
Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
ASCE7-10 SNOW LOAD DESIGN CRITERIA:
Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
Pm = 20 psf
HORIZONTAL REACTION(S) :
support 1 282 lb
support 2 282 lb

End verticals are designed for axial loads only unless noted otherwise. Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).
Required bearing widths and bearing areas apply when truss not supported in a hanger.
UPLIFT REACTION(S) :
Support C&C Wind Non-Wind
1 -198 lb
2 -320 lb
This truss is designed using the ASCE7-10 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = C
Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
Mean roof height = 24.30 ft, mph = 115
Occupancy Category II, Dead Load = 9.0 psf
Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding
Tributary Area = 50 sqft

TC	FORCE	AXL	BND	CSI
1-2	-2449	0.04	0.24	0.28
2-3	-4346	0.18	0.27	0.46
3-4	-2410	0.04	0.36	0.40
4-5	-1707	0.02	0.39	0.41
5-6	-635	0.00	0.22	0.22
6-7	-635	0.00	0.22	0.22

BC	FORCE	AXL	BND	CSI
8-9	2115	0.26	0.31	0.58
9-10	2433	0.30	0.09	0.40
10-11	2991	0.37	0.14	0.52
11-12	2362	0.29	0.10	0.39
12-13	1009	0.13	0.09	0.22
13-14	1010	0.13	0.12	0.25
14-15	283	0.00	0.11	0.11

WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	-1194	0.23	5-12	970	0.32
2-10	1684	0.56	5-13	171	0.06
3-10	1260	0.42	5-14	-895	0.37
3-11	-1024	0.44	6-14	-431	0.15
4-11	1556	0.52	7-14	1398	0.47
4-12	-1992	0.99	7-15	-1403	0.49



MAX DEFLECTION (span) :
L/999 MEM 4-5 (LIVE) LC 41
L= -0.25" CC= -0.07" CL= -0.31"

==== Joint Locations ====

1	0-0-0	9	2-2-8
2	2-5-8	10	4-2-8
3	6-2-15	11	10-7-0
4	10-7-0	12	12-7-0
5	16-3-1	13	16-3-1
6	20-4-5	14	20-4-5
7	24-9-0	15	24-9-0
8	0-0-0		

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 3/16" = 1'



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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.10
BC Live 0.00 psf	O.C.Spacing 2-0-0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301443

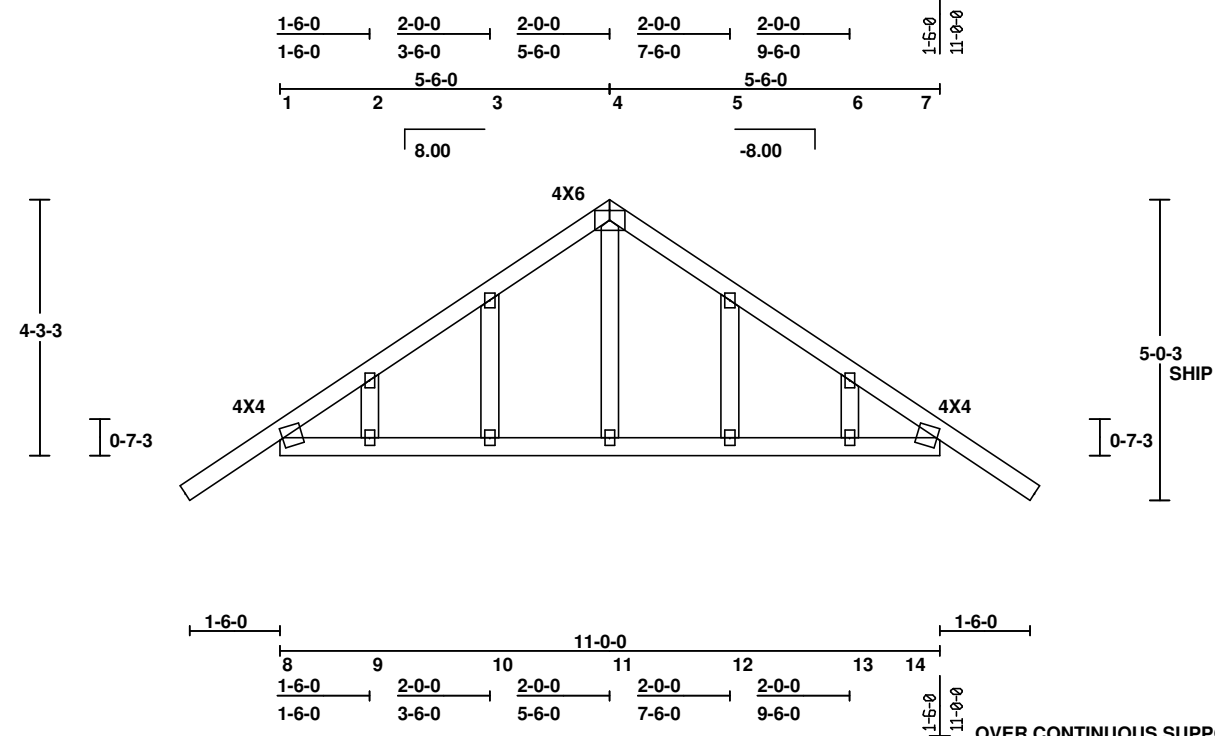
TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 GBL BLK 2x4 DFL STUD
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for
 Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on
 testing and approval as required by IBC 1703
 and ANSI/TPI and are reported in available
 documents as ER-1607 and ESR-1118.
 + + + + + OVERHANG(S) MAY BE SHORTENED UP TO 3" MAX.!
 Overhang Soffit loading = 0.0 psf
 + + + + + 20 psf bottom chord live load NOT required
 on this truss, per IBC/IRC requirements for
 attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the
 lesser of the truss chord lumber value or
 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Gable verticals are 2x 4 web material spaced
 at 24.0" o.c. unless noted otherwise.
 Top chord supports 24.0" of uniform load
 at 35 psf live load and 8 psf dead load.
 Additional design considerations may be
 required if sheathing is attached.
 Brace gable studs in accordance with
 Truswal Systems standard gable bracing
 details and charts.
 This truss requires adequate sheathing, as
 designed by others, applied to the truss
 face providing lateral support for webs in
 the truss plane and creating shear wall
 action to resist diaphragm loads.

This truss is designed using the
 ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.13 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 22 sqft

==== Joint Locations ====

1	0- 0- 0	8	0- 0- 0
2	1- 6- 0	9	1- 6- 0
3	3- 6- 0	10	3- 6- 0
4	5- 6- 0	11	5- 6- 0
5	7- 6- 0	12	7- 6- 0
6	9- 6- 0	13	9- 6- 0
7	11- 0- 0	14	11- 0- 0



4/15/2019

TYPICAL PLATE : 2X3

OVER CONTINUOUS SUPPORT

Scale: 5/16" = 1'

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P.O. BOX 787, BLACKFOOT, ID 83221

Eng. Job: .EJ. Chk: BJS Dsgnr: BJS	WO: FC BELMONT 2-1-30
TC Live 35.00 psf TC Dead 7.50 psf BC Live 0.00 psf BC Dead 7.50 psf TOTAL 50.00 psf	DurFacs L=1.15 P=1.15 Rep Mbr Bnd 1.15 O.C.Spacing 2- 0- 0 Design Spec IRC-2015 Seqn T6.5.21 - 301444

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	3586	5.50"	1.91"
2	10- 9- 4	4151	5.50"	2.21"

TC	FORCE	AXL	BND	CSI
1-2	-5174	0.06	0.24	0.30
2-3	-3948	0.03	0.14	0.18
3-4	-3955	0.04	0.12	0.16
4-5	-5094	0.06	0.17	0.22

BC	FORCE	AXL	BND	CSI
6-7	4173	0.17	0.22	0.39
7-8	4132	0.17	0.37	0.54
8-9	4093	0.17	0.18	0.34
9-10	4131	0.17	0.25	0.42

WEB	FORCE	CSI	WEB	FORCE	CSI
2-7	1518	0.25	4-8	-1298	0.16
2-8	-1266	0.16	4-9	1425	0.24
3-8	3992	0.67			

TC 2x4 DFL #1 & Btr.
 BC 2x6 DFL SS
 WEB 2x4 DFL STUD

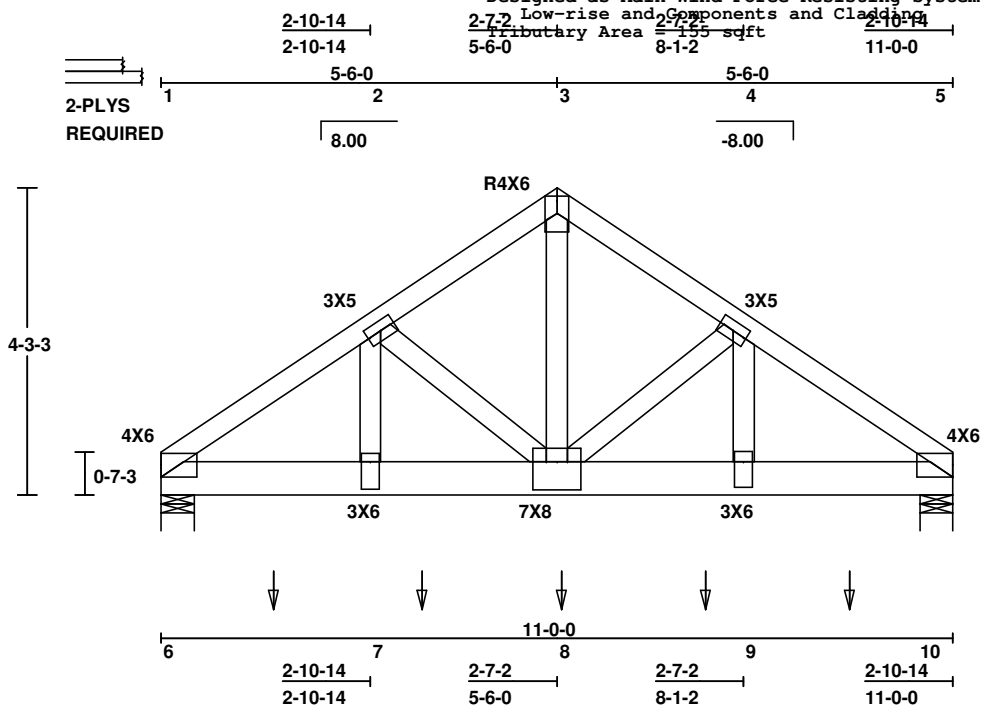
Lumber shear allowables are per NDS.
 Kcr (creep factor) = 2.00
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Permanent bracing is required (by others) to prevent rotation/toppling. See BCSI and ANSI/TPI 1.
 UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -1094 lb
 2 -892 lb

20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 2-PLY! Nail w/10d BOX, staggered (per NDS) in: TC- 2 BC- 2 WEBS- 2 **PER FOOT!**
 Cluster screws, if shown, are 3" long.
 HORIZONTAL REACTION(S) :
 support 1 111 lb

This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.13 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System

+++++
 Nail pattern shown is for PLF loads and point loads converted to PLF loads only. Concentrated loads MUST be distributed to each ply equally. Multi-ply with hangers are based on hanger nails using 1.5" nails min. into the carrying member.
 If shown, use additional fasteners for point loads as indicated from the back plys, distributed symmetrically around the hanger. Use any other approved detail (by others).
 10d = 10d NAILS, SDS = Simpson SDS screws or equivalent substitute
 (*) = Special Connection Req. (by others)
 @ = Indicates load is evenly distributed (no additional fasteners req.)
 + + + + +
 NAIL pattern shown is based on:
 10d BOX = 0.128" dia. x 3.0" long nail
 10d COMMON = 0.148" dia. x 3.0" long nail
 16d BOX = 0.135" dia. x 3.5" long nail
 16d COMMON = 0.162" dia. x 3.5" long nail
 + + + + +



-----LOAD CASE #1 DESIGN LOADS-----

Dir	L.Plf	L.Loc	R.Plf	R.Loc	LL/TL
TC Vert	85.00	0- 0- 0	85.00	11- 0- 0	0.42
BC Vert	15.00	0- 0- 0	15.00	0- 6-12	0.40
BC Vert	7.50	0- 6-12	7.50	2- 6-12	0.40
BC Vert	7.50	2- 7- 8	7.50	10- 6-12	0.40
BC Vert	15.00	10- 6-12	15.00	11- 0- 0	0.40

-----CRITICAL POINT LOADS-----

Type	X.Loc	Max/Dur	Min/Dur
BC Vert	1- 6-12	274/1.15	-85/1.60
BC Vert	3- 7- 8	2405/1.15	-1078/1.60
BC Vert	5- 6-12	1292/1.15	-231/1.60
BC Vert	7- 6-12	1403/1.15	-216/1.60
BC Vert	9- 6-12	1408/1.15	-198/1.60

MAX DEFLECTION (span) :
 L/999 MEM 7-8 (LIVE) LC 10
 L= -0.05" CC= -0.02" CL= -0.07"

==== Joint Locations ====

1	0- 0- 0	6	0- 0- 0
2	2-10-14	7	2-10-14
3	5- 6- 0	8	5- 6- 0
4	8- 1- 2	9	8- 1- 2
5	11- 0- 0	10	11- 0- 0

4/15/2019

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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.00
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301445

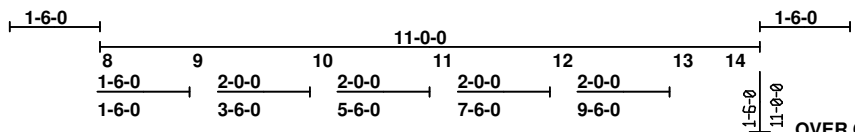
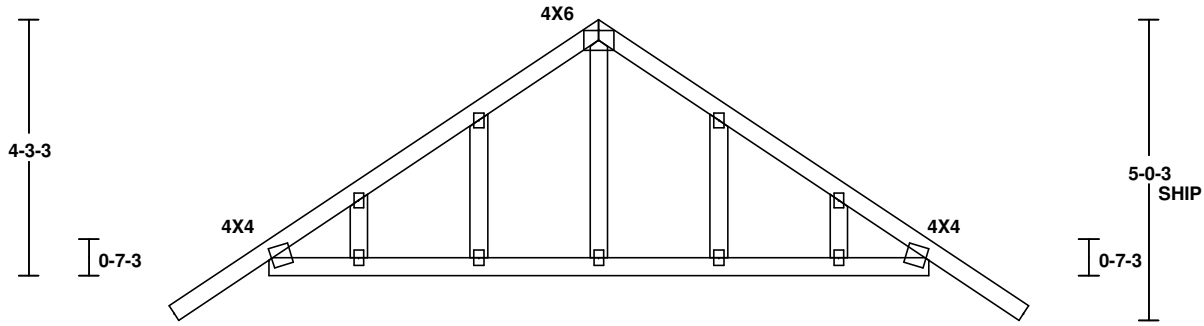
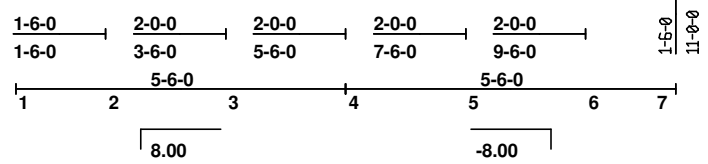
TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 GBL BLK 2x4 DFL STUD
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for
 Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on
 testing and approval as required by IBC 1703
 and ANSI/TPI and are reported in available
 documents as ER-1607 and ESR-1118.
 20 psf bottom chord live load NOT required
 on this truss, per IBC/IRC requirements for
 attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the
 lesser of the truss chord lumber value or
 625 for all bearings.
**THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.**
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Gable verticals are 2x 4 web material spaced
 at 24.0" o.c. unless noted otherwise.
 Top chord supports 24.0" of uniform load
 at 35 psf live load and 8 psf dead load.
 Additional design considerations may be
 required if sheathing is attached.
 Brace gable studs in accordance with
 Truswal Systems standard gable bracing
 details and charts.
 This truss requires adequate sheathing, as
 designed by others, applied to the truss
 face providing lateral support for webs in
 the truss plane and creating shear wall
 action to resist diaphragm loads.

This truss is designed using the
 ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.13 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 22 sqft

==== Joint Locations ====

1	0- 0- 0	8	0- 0- 0
2	1- 6- 0	9	1- 6- 0
3	3- 6- 0	10	3- 6- 0
4	5- 6- 0	11	5- 6- 0
5	7- 6- 0	12	7- 6- 0
6	9- 6- 0	13	9- 6- 0
7	11- 0- 0	14	11- 0- 0



TYPICAL PLATE : 2X3

4/15/2019

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Scale: 5/16" = 1'

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P.O. BOX 787, BLACKFOOT, ID 83221

Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.15
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301446

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	3714	5.50"	1.98"
2	10- 9- 4	5441	5.50"	2.90"

TC	FORCE	AXL	BND	CSI
1-2	-5214	0.06	0.26	0.32
2-3	-4547	0.04	0.14	0.18
3-4	-4552	0.05	0.10	0.15
4-5	-6041	0.08	0.17	0.26

BC	FORCE	AXL	BND	CSI
6-7	4198	0.16	0.27	0.43
7-8	4177	0.16	0.41	0.57
8-9	4860	0.19	0.26	0.45
9-10	4909	0.19	0.31	0.51

WEB	FORCE	CSI	WEB	FORCE	CSI
2-7	837	0.14	4-8	-1480	0.19
2-8	-687	0.09	4-9	1768	0.30
3-8	4622	0.77			

TC 2x4 DFL #1 & Btr.
 BC 2x6 DFL SS
 WEB 2x4 DFL STUD
 PLT BLK 2x4 DFL #1 & Btr.
 Lumber shear allowables are per NDS.
 Refer to Joint QC Detail Sheet for
 Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on
 testing and approval as required by IBC 1703
 and ANSI/TPI and are reported in available
 documents as ER-1607 and ESR-1118.
 Permanent bracing is required (by others) to
 prevent rotation/toppling. See BCSI
 and ANSI/TPI 1.
 UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -1182 lb
 2 -1328 lb
 20 psf bottom chord live load NOT required
 on this truss, per IRC/IBC requirements for
 attics with limited storage.

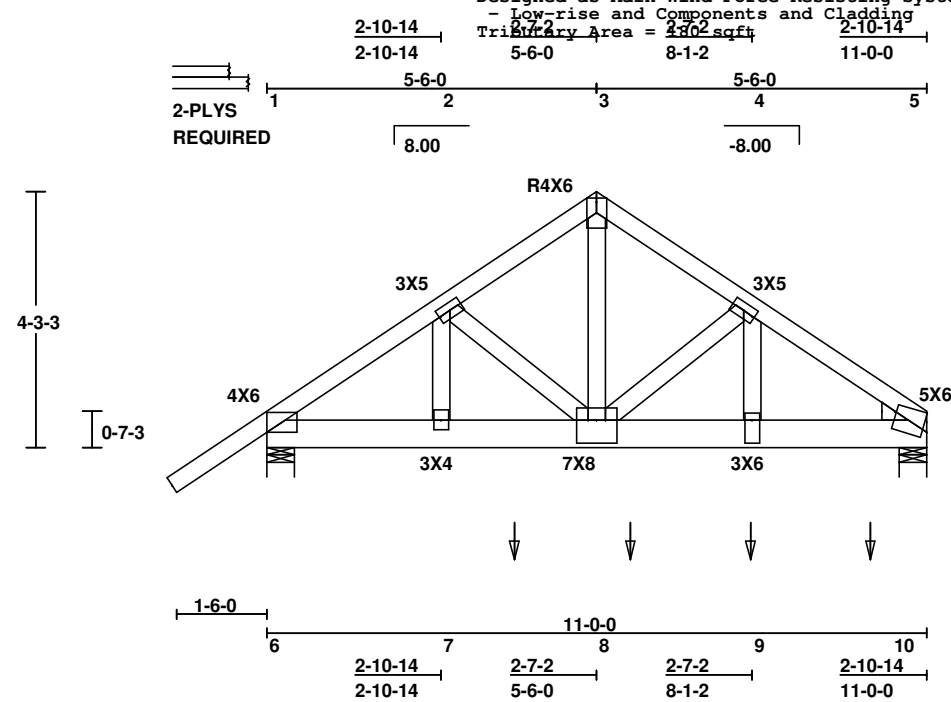
Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the
 lesser of the truss chord lumber value or
 625 for all bearings.
 Kcr (creep factor) = 2.00
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 2-Ply! Nail w/10d BOX, staggered (per NDS)
 in: TC- 2 BC- 2 WEBS- 2 **PER FOOT!**
 Cluster screws, if shown, are 3" long.
 HORIZONTAL REACTION(S) :
 support 1 111 lb
 This truss is designed using the
 ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.13 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Triangular Area = 280 sqft

+++++
 Nail pattern shown is for PLF loads and
 point loads converted to PLF loads only.
 Concentrated loads MUST be distributed to
 each ply equally. Multi-ply with hangers are
 based on hanger nails using 1.5" nails min.
 into the carrying member.
 If shown, use additional fasteners for point
 loads as indicated from the back plys,
 distributed symmetrically around the hanger.
 Use any other approved detail (by others).
 10d = 10d NAILS, SDS = Simpson SDS screws
 or equivalent substitute
 (*) = Special Connection Req. (by others)
 @ = Indicates load is evenly distributed
 (no additional fasteners req.)
 +++++
 Nail pattern shown is based on:
 10d BOX = 0.128" dia. x 3.0" long nail
 10d COMMON = 0.148" dia. x 3.0" long nail
 16d BOX = 0.135" dia. x 3.5" long nail
 16d COMMON = 0.162" dia. x 3.5" long nail
 +++++
 -----LOAD CASE #1 DESIGN LOADS-----

Dir	L.Plf	L.Loc	R.Plf	R.Loc	LL/TL
TC Vert	100.00	- 1- 6- 0	100.00	0- 0- 0	0.10
TC Vert	85.00	0- 0- 0	85.00	11- 0- 0	0.42
BC Vert	15.00	0- 0- 0	15.00	3- 1- 8	0.40
BC Vert	7.50	3- 1- 8	7.50	11- 0- 0	0.00

...Type... lbs X.Loc LL/TL
 TC Vert 60.0 - 1- 6- 0 1.00
 -----CRITICAL POINT LOADS-----

Type	X.Loc	Max/Dur	Min/Dur
BC Vert	4- 1- 8	2537/1.15	-1226/1.60
BC Vert	6- 0-12	1774/1.15	-350/1.60
BC Vert	8- 0-12	1774/1.15	-355/1.60
BC Vert	10- 0-12	1905/1.15	-360/1.60



MAX DEFLECTION (span) :
 L/999 MEM 7-8 (LIVE) LC 1
 L= -0.06" CC= -0.02" CL= -0.08"

==== Joint Locations =====

1	0- 0- 0	6	0- 0- 0
2	2-10-14	7	2-10-14
3	5- 6- 0	8	5- 6- 0
4	8- 1- 2	9	8- 1- 2
5	11- 0- 0	10	11- 0- 0

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted. Scale: 5/16" = 1'



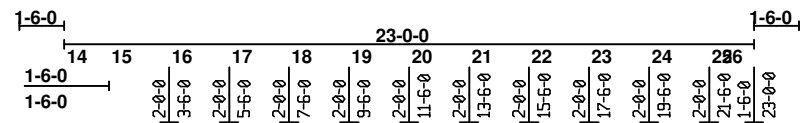
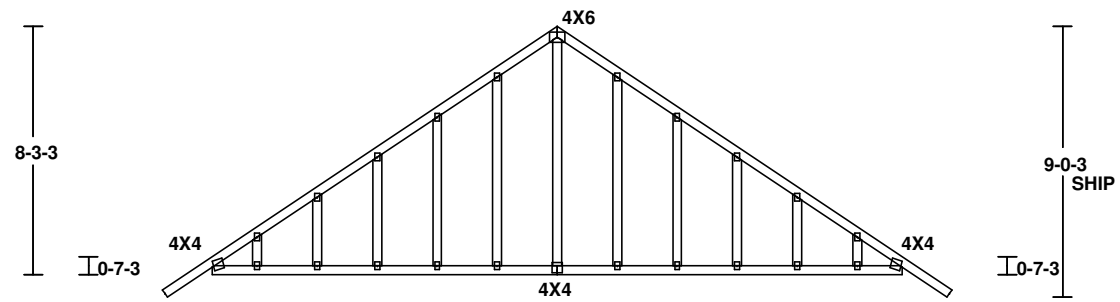
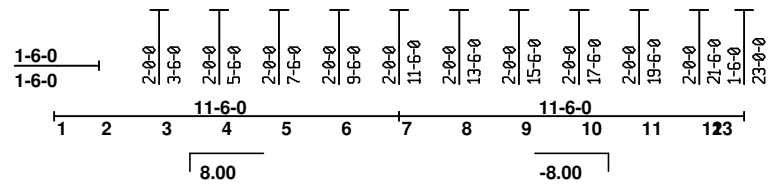
WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.00
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301447

TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 GBL BLK 2x4 DFL STUD
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for
 Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on
 testing and approval as required by IBC 1703
 and ANSI/TPI and are reported in available
 documents as ER-1607 and ESR-1118.
 20 psf Live Load applied to bottom chord in
 accordance with IRC/IBC requirements for
 attics with limited storage having a clear
 height >= 42" and clear width of >= 24".

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the
 lesser of the truss chord lumber value or
 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Gable verticals are 2x 4 web material spaced
 at 24.0" o.c. unless noted otherwise.
 Top chord supports 24.0" of uniform load
 at 35 psf live load and 8 psf dead load.
 Additional design considerations may be
 required if sheathing is attached.
 Brace gable studs in accordance with
 Truswal Systems standard gable bracing
 details and charts.
 This truss requires adequate sheathing, as
 designed by others, applied to the truss
 face providing lateral support for webs in
 the truss plane and creating shear wall
 action to resist diaphragm loads.

This truss is designed using the
 ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 24.13 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 46 sqft



==== Joint Locations ====

1	0- 0- 0	14	0- 0- 0
2	1- 6- 0	15	1- 6- 0
3	3- 6- 0	16	3- 6- 0
4	5- 6- 0	17	5- 6- 0
5	7- 6- 0	18	7- 6- 0
6	9- 6- 0	19	9- 6- 0
7	11- 6- 0	20	11- 6- 0
8	13- 6- 0	21	13- 6- 0
9	15- 6- 0	22	15- 6- 0
10	17- 6- 0	23	17- 6- 0
11	19- 6- 0	24	19- 6- 0
12	21- 6- 0	25	21- 6- 0
13	23- 0- 0	26	23- 0- 0

TYPICAL PLATE : 2X3

OVER CONTINUOUS SUPPORT

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

4/15/2019

Scale: 5/32" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301448

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0-1-12	1360	3.50"	1.50"	2x4 DFL #1 & Btr.
2	22-10-3	1150	3.62"	1.50"	2x4 DFL #1 & Btr.

TC	FORCE	AXL	BND	CSI
1-2	-1577	0.02	0.28	0.30
2-3	-1147	0.01	0.28	0.28
3-4	-1147	0.01	0.28	0.28
4-5	-1576	0.02	0.28	0.30

BC	FORCE	AXL	BND	CSI
6-7	1205	0.15	0.23	0.38
7-8	1204	0.15	0.11	0.26
8-9	1203	0.15	0.11	0.26
9-10	1203	0.15	0.24	0.39

WEB	FORCE	CSI	WEB	FORCE	CSI
2-7	180	0.06	4-8	-450	0.39
2-8	-451	0.39	4-9	179	0.06
3-8	652	0.22			

WEDGE 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

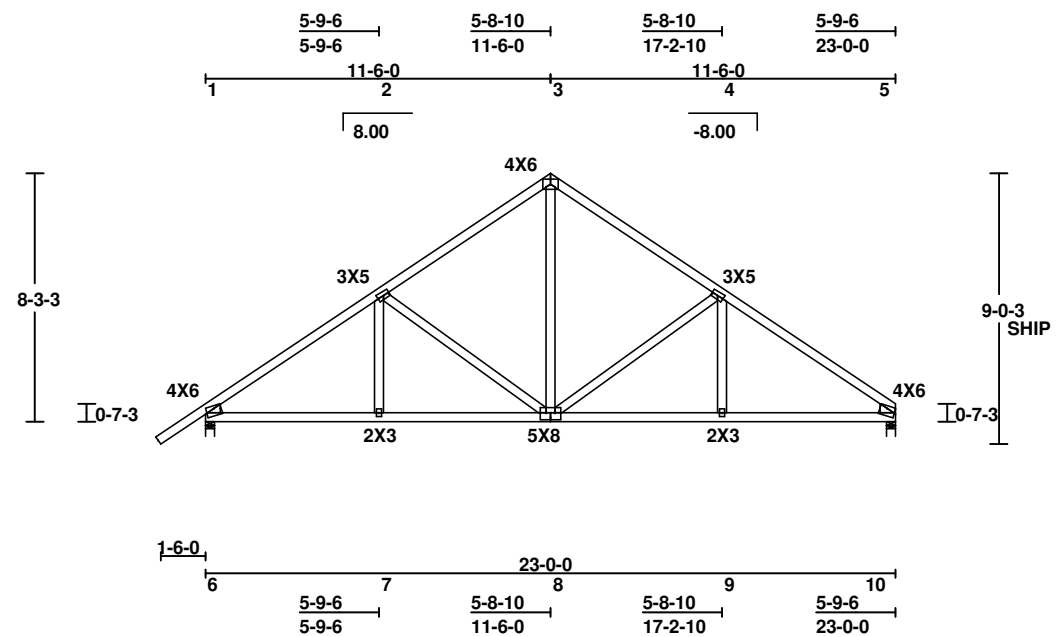
[PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -269 lb
 2 -203 lb
 HORIZONTAL REACTION(S) :
 support 1 243 lb
 support 2 -243 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 24.13 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 46 sqft

MAX DEFLECTION (span) :
 L/999 MEM 3-4 (LIVE) LC 1
 L= -0.13" CC= -0.04" CL= -0.17"

==== Joint Locations =====

1	0- 0- 0	6	0- 0- 0
2	5- 9- 6	7	5- 9- 6
3	11- 6- 0	8	11- 6- 0
4	17- 2-10	9	17- 2-10
5	23- 0- 0	10	23- 0- 0



4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted. Scale: 5/32" = 1'



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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.15
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301449

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-0-12	1288	HGR	1.50"
2	22-10-4	2447	3.50"	2.61"

BRG HANGER/CLIP NOTE
1 *HUS26

Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC 2x4 DFL #1 & Btr.
BC 2x4 DFL #1 & Btr.
WEB 2x4 DFL STUD
WEDGE 2x4 DFL #1 & Btr.
PLT BLK 2x4 DFL #1 & Btr.
GBL BLK 2x4 DFL STUD

Kcr (creep factor) = 2.00
Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
Gable verticals are 2x 4 web material spaced at 24.0" o.c. unless noted otherwise.
Top chord supports 24.0" of uniform load at 35 psf live load and 8 psf dead load.
Additional design considerations may be required if sheathing is attached.
Brace gable studs in accordance with Truswal Systems standard gable bracing details and charts.
This truss requires adequate sheathing, as designed by others, applied to the truss face providing lateral support for webs in the truss plane and creating shear wall action to resist diaphragm loads.
Right Overhang(s) are not to be removed.
OVERHANG(S) MAY BE SHORTENED UP TO 9'-0" MAX!
Right Overhang Soffit load = 09'-0" @ 10 psf

Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details.
Designed per ANSI/TPI 1-2014
Fabrication Tolerance = 20.0%
Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
ASCE7-10 SNOW LOAD DESIGN CRITERIA:
Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.
20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

Required bearing widths and bearing areas apply when truss not supported in a hanger.
UPLIFT REACTION(S) :
Support C&C Wind Non-Wind
1 -184 lb
2 -485 lb
HORIZONTAL REACTION(S) :
support 1 -326 lb
support 2 -326 lb
This truss is designed using the ASCE7-10 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = C
Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
Mean roof height = 25.12 ft, mph = 115
Occupancy Category II, Dead Load = 9.0 psf
Designed as Main Wind Force Resisting System
- Low-rise and Components and Cladding
Tributary Area = 58 sqft

TC	FORCE	AXL	BND	CSI
1-2	-1800	0.02	0.24	0.27
2-3	-1665	0.02	0.24	0.25
3-4	-911	0.01	0.24	0.24
4-5	-911	0.00	0.22	0.23
5-6	892	0.08	0.30	0.38
6-7	763	0.10	0.31	0.40
7-OR	75	0.01	0.16	0.17

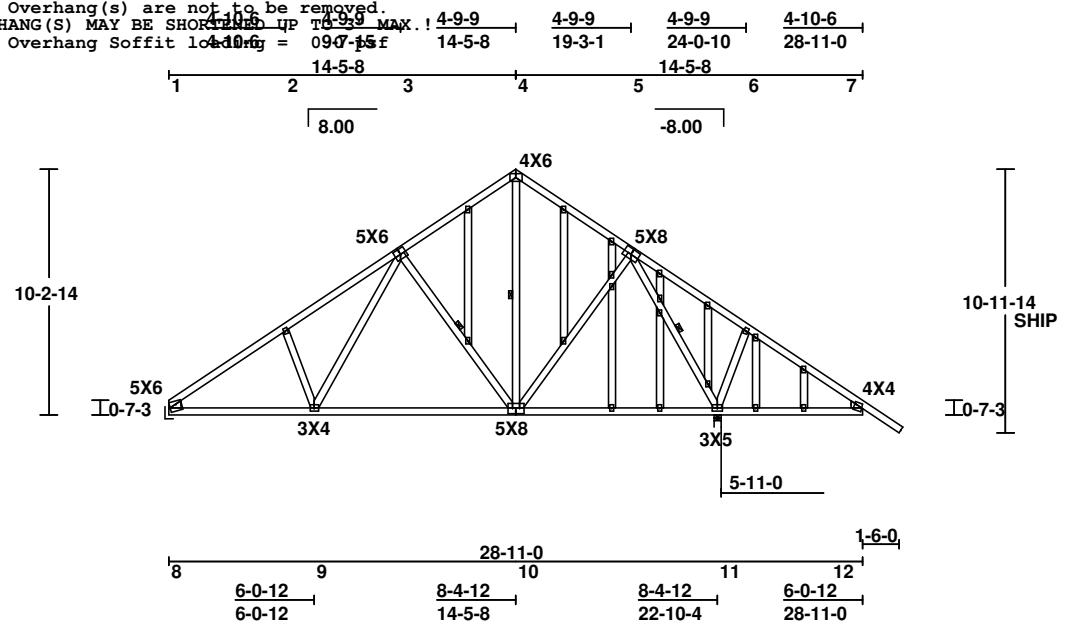
BC	FORCE	AXL	BND	CSI
8-9	1401	0.16	0.27	0.43
9-10	1011	0.12	0.50	0.62
10-11	565	0.03	0.50	0.53
11-12	-521	0.00	0.46	0.46

WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	-311	0.09	5-10	602	0.20
3-9	621	0.21	5-11	-1964	0.95
3-10	-667	0.24	6-11	-396	0.12
4-10	498	0.22			

MAX DEFLECTION (span) :
L/999 MEM 9-10 (LIVE) LC 47
L= -0.19" CC= -0.07" CL= -0.26"
MAX DEFLECTION (cant) :
L/481 MEM 11-12 (LIVE) LC 52
L= 0.15" CC= -0.02" CL= 0.13"

==== Joint Locations ====

1	0-0-0	7	28-11-0
2	4-10-6	8	0-0-0
3	9-7-15	9	6-0-12
4	14-5-8	10	14-5-8
5	19-3-1	11	22-10-4
6	24-0-10	12	28-11-0



TYPICAL PLATE : 2X3

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

4/15/2019

Scale: 1/8" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2-0-0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301450

BRG	X-LOC	REACT	SIZE	REQ'D
1	0-0-12	1397	HGR	1.50"
2	22-10-4	1915	3.50"	2.04"
3	28-2-12	346	5.50"	1.50"

TC	2x4	DFL	#1 & Btr.
BC	2x4	DFL	#1 & Btr.
WEB	2x4	DFL	STUD
WEDGE	2x4	DFL	#1 & Btr.
SLIDER	2x6	DFL	SS

Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details. **[PM]=PLATE MONITOR USED-See Joint Report** Designed per ANSI/TPI 1-2014 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings. THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES. Loaded for 10 PSF non-concurrent BCLL. ASCE7-10 SNOW LOAD DESIGN CRITERIA: Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00 Install interior support(s) before erection. 20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

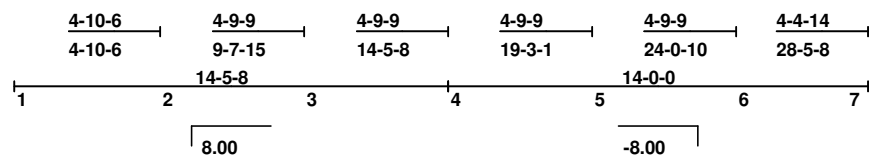
Required bearing widths and bearing areas apply when truss not supported in a hanger. UPLIFT REACTION(S) : Support C&C Wind Non-Wind
 1 -209 lb
 2 -192 lb
 3 -97 lb -49 lb
 HORIZONTAL REACTION(S) : support 1 307 lb support 3 307 lb
 This truss is designed using the ASCE7-10 Wind Specification Bldg Enclosed = Yes, Truss Location = Not End Zone Exp Category = C Bldg Length = 99.00 ft, Bldg Width = 50.00 ft Mean roof height = 25.12 ft, mph = 115 Occupancy Category II, Dead Load = 9.0 psf Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding Tributary Area = 57 sqft

BRG HANGER/CLIP NOTE
 1 *HUS26
 Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC	FORCE	AXL	BND	CSI
1-2	-1978	0.03	0.27	0.30
2-3	-1841	0.02	0.24	0.26
3-4	-1056	0.01	0.24	0.25
4-5	-1054	0.00	0.22	0.22
5-6	304	0.03	0.27	0.30
6-7	-244	0.01	0.30	0.31
6-7	-52	0.00	0.10	0.10

BC	FORCE	AXL	BND	CSI
8-9	1547	0.19	0.25	0.45
9-10	1167	0.15	0.51	0.65
10-11	577	0.07	0.51	0.58
11-12	-289	0.00	0.36	0.36

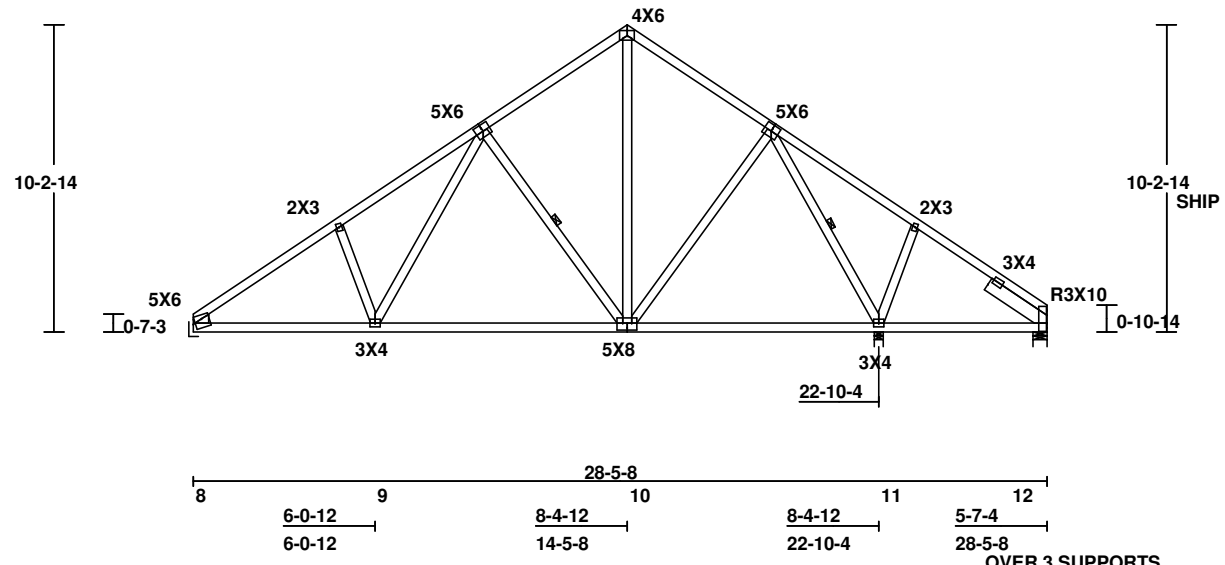
WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	-304	0.09	5-10	403	0.13
3-9	604	0.20	5-11	-1436	0.64
3-10	-660	0.23	6-11	-355	0.10
4-10	645	0.32	SLDRR	-278	0.02



MAX DEFLECTION (span) :
 L/999 MEM 9-10 (LIVE) LC 43
 L= -0.20" CC= -0.07" CL= -0.27"

==== Joint Locations ====

1	0-0-0	7	28-5-8
2	4-10-6	8	0-0-0
3	9-7-15	9	6-0-12
4	14-5-8	10	14-5-8
5	19-3-1	11	22-10-4
6	24-0-10	12	28-5-8



4/15/2019

OVER 3 SUPPORTS

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 5/32" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2-0-0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301451

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 0-12	1412	HGR	1.51"
2	22- 8-12	1476	5.50"	1.57"

BRG HANGER/CLIP NOTE
1 *HUS26

Support Connection(s)/Hanger(s) are not designed for horizontal loads. SEE SIMPSON CATALOG FOR ADDITIONAL INSTALLATION NOTES

TC 2x4 DFL #1 & Btr.
BC 2x4 DFL #1 & Btr.
WEB 2x4 DFL STUD
PLT BLK 2x4 DFL #1 & Btr.

Fabrication Tolerance = 20.0%
Kcr (creep factor) = 2.00
Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used

IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.

End verticals are designed for axial loads only unless noted otherwise.
Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).

Web bracing required at each location shown. Refer to BCSI for proper required lateral restraint. For alternative web bracing, see ITWBCG's standard details.
Designed per ANSI/TPI 1-2014
Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
ASCE7-10 SNOW LOAD DESIGN CRITERIA:
Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
Hanger nail points that protrude through the back face of the girder shall be clinched or covered for safety.
20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

Required bearing widths and bearing areas apply when truss not supported in a hanger.

UPLIFT REACTION(S) :
Support C&C Wind Non-Wind
1 -189 lb
2 -224 lb

HORIZONTAL REACTION(S) :
support 1 262 lb
support 2 262 lb

This truss is designed using the ASCE7-10 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = C
Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
Mean roof height = 25.12 ft, mph = 115
Occupancy Category II, Dead Load = 9.0 psf
Designed as Main Wind Force Resisting System
- Low-rise and Components and Cladding
Tributary Area = 46 sqft

TC	FORCE	AXL	BND	CSI
1-2	-2047	0.03	0.33	0.36
2-3	-1896	0.02	0.23	0.25
3-4	-1098	0.01	0.23	0.24
4-5	-1093	0.01	0.20	0.21
5-6	104	0.01	0.19	0.20

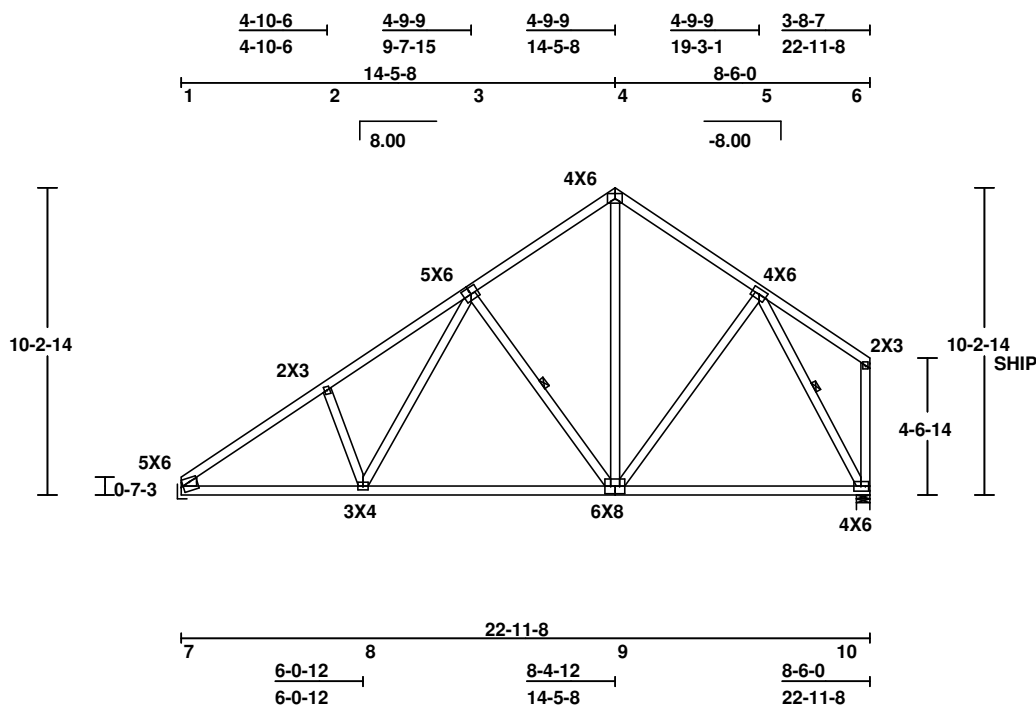
BC	FORCE	AXL	BND	CSI
7-8	1595	0.20	0.23	0.43
8-9	1203	0.15	0.51	0.66
9-10	616	0.08	0.54	0.62

WEB	FORCE	CSI	WEB	FORCE	CSI
2-8	-316	0.09	5-9	368	0.12
3-8	620	0.21	5-10	-1356	0.67
3-9	-661	0.23	6-10	-122	0.04
4-9	685	0.25			

MAX DEFLECTION (span) :
L/999 MEM 9-10 (LIVE) LC 43
L= -0.19" CC= -0.08" CL= -0.27"

==== Joint Locations =====

1	0- 0- 0	6	22-11- 8
2	4-10- 6	7	0- 0- 0
3	9- 7-15	8	6- 0-12
4	14- 5- 8	9	14- 5- 8
5	19- 3- 1	10	22-11- 8



4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 5/32" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301452

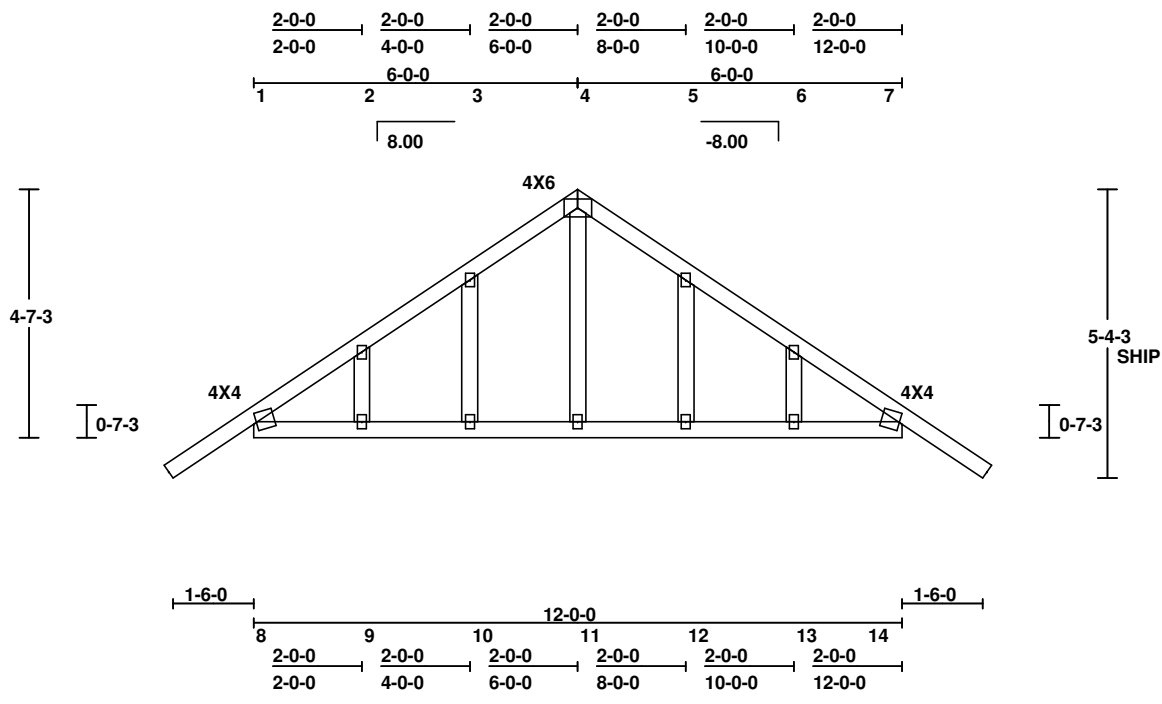
TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 GBL BLK 2x4 DFL STUD
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for
 Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on
 testing and approval as required by IBC 1703
 and ANSI/TPI and are reported in available
 documents as ER-1607 and ESR-1118.
 20 psf bottom chord live load NOT required
 on this truss, per IBC/IRC requirements for
 attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the
 lesser of the truss chord lumber value or
 625 for all bearings.
**THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.**
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Gable verticals are 2x 4 web material spaced
 at 24.0 " o.c. unless noted otherwise.
 Top chord supports 24.0 " of uniform load
 at 35 psf live load and 8 psf dead load.
 Additional design considerations may be
 required if sheathing is attached.
 Brace gable studs in accordance with
 Truswal Systems standard gable bracing
 details and charts.
 This truss requires adequate sheathing, as
 designed by others, applied to the truss
 face providing lateral support for webs in
 the truss plane and creating shear wall
 action to resist diaphragm loads.

This truss is designed using the
 ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.30 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 24 sqft

==== Joint Locations =====

1	0-0-0	8	0-0-0
2	2-0-0	9	2-0-0
3	4-0-0	10	4-0-0
4	6-0-0	11	6-0-0
5	8-0-0	12	8-0-0
6	10-0-0	13	10-0-0
7	12-0-0	14	12-0-0



4/15/2019

OVER CONTINUOUS SUPPORT

Scale: 9/32" = 1'

TYPICAL PLATE : 2X3

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.



P.O. BOX 787, BLACKFOOT, ID 83221

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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.15
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301453

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	4196	5.50"	2.24"
2	11- 9- 3	5053	5.62"	2.69"

TC	FORCE	AXL	BND	CSI
1-2	-6033	0.09	0.30	0.39
2-3	-5003	0.06	0.15	0.21
3-4	-5007	0.06	0.16	0.22
4-5	-6609	0.10	0.23	0.33

BC	FORCE	AXL	BND	CSI
6-7	4871	0.20	0.25	0.45
7-8	4840	0.20	0.43	0.63
8-9	5319	0.22	0.24	0.46
9-10	5371	0.22	0.29	0.51

WEB	FORCE	CSI	WEB	FORCE	CSI
2-7	1222	0.20	4-8	-1610	0.22
2-8	-1034	0.14	4-9	1920	0.32
3-8	5098	0.85			

TC 2x4 DFL #1 & Btr.
 BC 2x6 DFL SS
 WEB 2x4 DFL STUD
 PLT BLK 2x4 DFL #1 & Btr.

Lumber shear allowables are per NDS.
 Refer to Joint QC Detail Sheet for
 Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on
 testing and approval as required by IBC 1703
 and ANSI/TPI and are reported in available
 documents as ER-1607 and ESR-1118.
 Permanent bracing is required (by others) to
 prevent rotation/toppling. See BCSI
 and ANSI/TPI 1.

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -1310 lb
 2 -1221 lb

20 psf bottom chord live load NOT required
 on this truss, per IBC/IRC requirements for
 attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the
 lesser of the truss chord lumber value or
 625 for all bearings.
 Kcr (creep factor) = 2.00
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 2-PLY! Nail w/10d BOX, staggered (per NDS)
 in: TC- 2 BC- 2 WEBS- 2 **PER FOOT!**
 Cluster screws, if shown, are 3" long.

HORIZONTAL REACTION(S) :
 support 1 122 lb

This truss is designed using the
 ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.30 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 185 sqft

+++++
 Nail pattern shown is for PLF loads and
 point loads converted to PLF loads only.
 Concentrated loads MUST be distributed to
 each ply equally. Multi-ply with hangers are
 based on hanger nails using 1.5" nails min.
 into the carrying member.
 If shown, use additional fasteners for point
 loads as indicated from the back plys,
 distributed symmetrically around the hanger.
 Use any other approved detail (by others).
 10d = 10d NAILS, SDS = Simpson SDS screws
 or equivalent substitute
 (*) = Special Connection Req. (by others)
 @ = Indicates load is evenly distributed
 (no additional fasteners req.)
 + + + + +
 NAIL pattern shown is based on:
 10d BOX = 0.128" dia. x 3.0" long nail
 10d COMMON = 0.148" dia. x 3.0" long nail
 16d BOX = 0.135" dia. x 3.5" long nail
 16d COMMON = 0.162" dia. x 3.5" long nail
 + + + + +

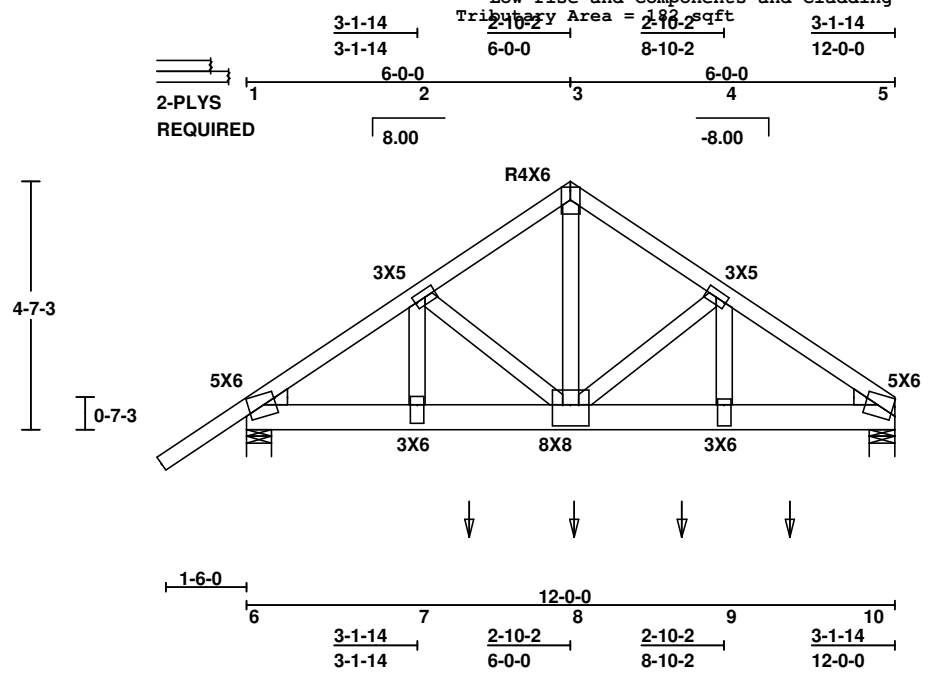
-----LOAD CASE #1 DESIGN LOADS-----

Dir	L.Plf	L.Loc	R.Plf	R.Loc	LL/TL
TC Vert	100.00	- 1- 6- 0	100.00	0- 0- 0	0.10
TC Vert	85.00	0- 0- 0	85.00	12- 0- 0	0.42
BC Vert	15.00	0- 0- 0	15.00	3- 1- 8	0.00
BC Vert	7.50	3- 1- 8	7.50	11- 0- 12	0.00
BC Vert	15.00	11- 0- 12	15.00	12- 0- 0	0.00

...Type... lbs X.Loc LL/TL
 TC Vert 60.0 - 1- 6- 0 1.00

----- CRITICAL POINT LOADS -----

Type	X.Loc	Max/Dur	Min/Dur
BC Vert	4- 1- 8	2542/1.15	-1249/1.60
BC Vert	6- 0- 12	1774/1.15	-350/1.60
BC Vert	8- 0- 12	1774/1.15	-355/1.60
BC Vert	10- 0- 12	1905/1.15	-360/1.60



MAX DEFLECTION (span) :
 L/999 MEM 7-8 (LIVE) LC 1
 L= -0.07" CC= -0.03" CL= -0.10"

==== Joint Locations =====

1	0- 0- 0	6	0- 0- 0
2	3- 1- 14	7	3- 1- 14
3	6- 0- 0	8	6- 0- 0
4	8- 10- 2	9	8- 10- 2
5	12- 0- 0	10	12- 0- 0

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted. Scale: 9/32" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.00
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301454

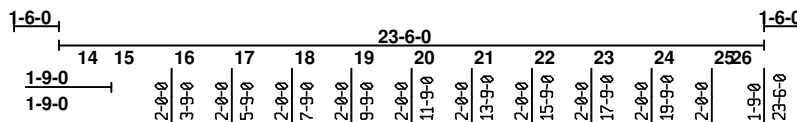
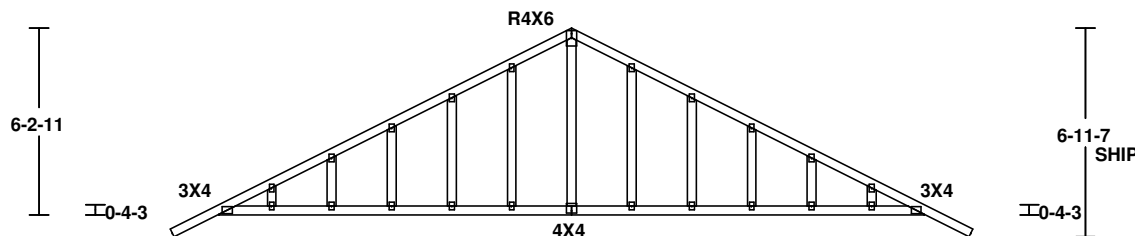
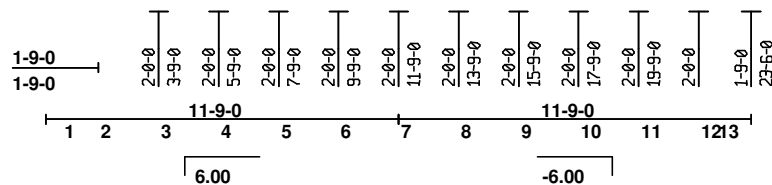
TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 GBL BLK 2x4 DFL STUD
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Gable verticals are 2x 4 web material spaced at 24.0" o.c. unless noted otherwise.
 Top chord supports 24.0" of uniform load at 35 psf live load and 8 psf dead load. Additional design considerations may be required if sheathing is attached.
 Brace gable studs in accordance with Truswal Systems standard gable bracing details and charts.
 This truss requires adequate sheathing, as designed by others, applied to the truss face providing lateral support for webs in the truss plane and creating shear wall action to resist diaphragm loads.

[PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 20 psf Live Load applied to bottom chord in accordance with IBC/IRC requirements for attics with limited storage having a clear height >= 42" and clear width of >= 24".

This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 23.11 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 47 sqft

==== Joint Locations ====

1	0-0-0	14	0-0-0
2	1-9-0	15	1-9-0
3	3-9-0	16	3-9-0
4	5-9-0	17	5-9-0
5	7-9-0	18	7-9-0
6	9-9-0	19	9-9-0
7	11-9-0	20	11-9-0
8	13-9-0	21	13-9-0
9	15-9-0	22	15-9-0
10	17-9-0	23	17-9-0
11	19-9-0	24	19-9-0
12	21-9-0	25	21-9-0
13	23-6-0	26	23-6-0



TYPICAL PLATE : 2X3

OVER CONTINUOUS SUPPORT

4/15/2019

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Scale: 5/32" = 1'

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P.O. BOX 787, BLACKFOOT, ID 83221

Eng. Job: .EJ.
 Chk: BJS
 Dsgnr: BJS

TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO: FC BELMONT 2-1-30

DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301455

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0- 1-12	1385	3.50"	1.50"	2x4 DFL #1 & Btr.
2	23- 4- 4	1385	3.50"	1.50"	2x4 DFL #1 & Btr.
					2x4 DFL STUD

TC	FORCE	AXL	BND	CSI
1-2	-2220	0.03	0.44	0.47
2-3	-1444	0.02	0.47	0.48
3-4	-1444	0.02	0.47	0.48
4-5	-2220	0.03	0.44	0.47

BC	FORCE	AXL	BND	CSI
6-7	1901	0.22	0.10	0.32
7-8	1899	0.24	0.07	0.31
8-9	1899	0.24	0.07	0.31
9-10	1901	0.22	0.10	0.32

WEB	FORCE	CSI	WEB	FORCE	CSI
2-7	206	0.06	4-8	-887	0.64
2-8	-887	0.64	4-9	206	0.06
3-8	736	0.25			

Lumber shear allowables are per NDS.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

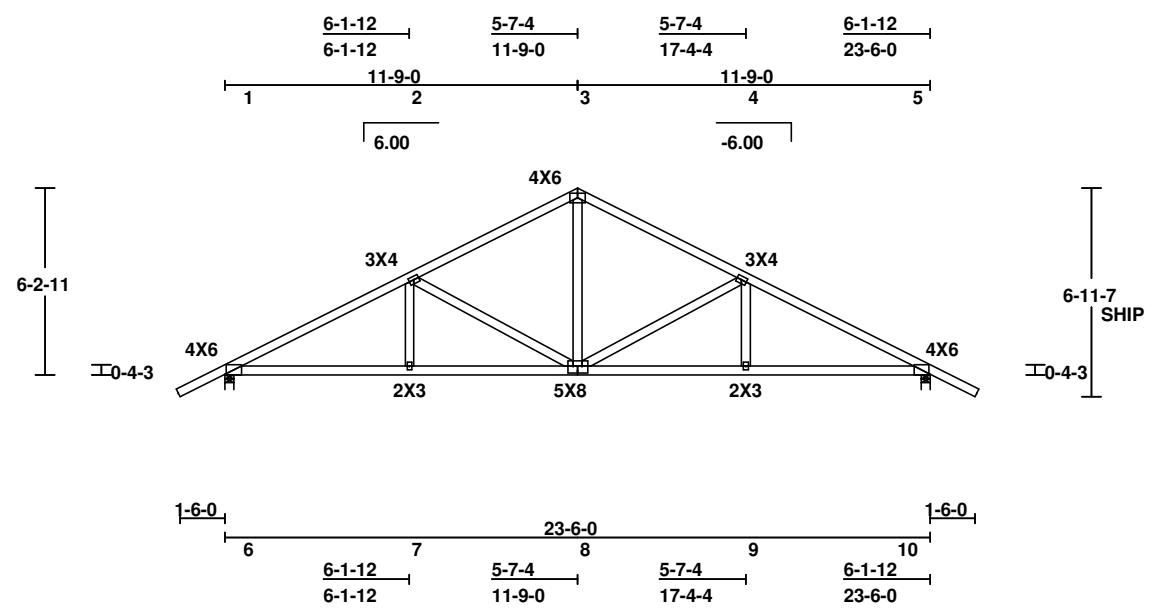
[PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -321 lb
 2 -321 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 23.11 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 47 sqft

MAX DEFLECTION (span) :
 L/999 MEM 2-3 (LIVE) LC 41
 L= -0.20" CC= -0.04" CL= -0.24"

==== Joint Locations =====

1	0- 0- 0	6	0- 0- 0
2	6- 1-12	7	6- 1-12
3	11- 9- 0	8	11- 9- 0
4	17- 4- 4	9	17- 4- 4
5	23- 6- 0	10	23- 6- 0



4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted. Scale: 5/32" = 1'



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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.15
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301456

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0- 1-12	7348	3.50"	2.61"	2x4 DFL #1 & Btr.
2	23- 4- 4	8357	3.50"	2.97"	2x6 DFL SS
					2x4 DFL STUD

TC	FORCE	AXL	BND	CSI
1-2	-14381	0.26	0.15	0.41
2-3	-12148	0.18	0.09	0.27
3-4	-9468	0.07	0.08	0.15
4-5	-9467	0.07	0.08	0.15
5-6	-12527	0.18	0.08	0.26
6-7	-15395	0.29	0.22	0.51

BC	FORCE	AXL	BND	CSI
8-9	12808	0.31	0.20	0.51
9-10	12740	0.31	0.16	0.47
10-11	10621	0.26	0.13	0.39
11-12	10928	0.27	0.12	0.39
12-13	13643	0.34	0.14	0.47
13-14	13732	0.32	0.25	0.57

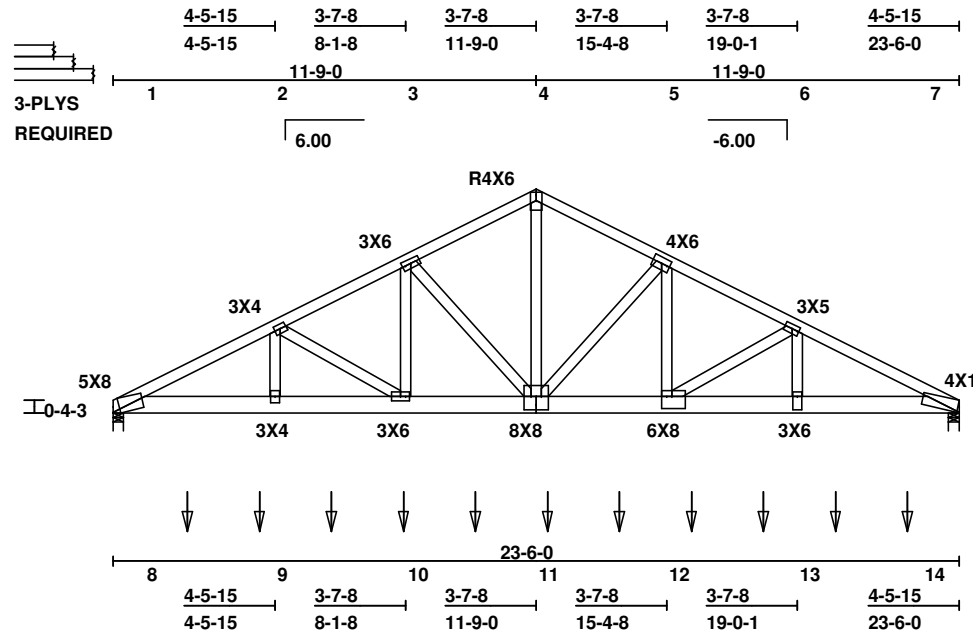
WEB	FORCE	CSI	WEB	FORCE	CSI
2-9	2137	0.24	5-11	-4069	0.48
2-10	-2519	0.28	5-12	4185	0.47
3-10	3620	0.40	6-12	-3187	0.35
3-11	-3594	0.42	6-13	2738	0.31
4-11	8023	0.89			

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
3-PLY! Nail w/10d BOX, staggered (per NDS) in: TC- 2 BC- 2 WEBS- 2 **PER FOOT!**
 Nail in layers or from each face.
 Cluster screws, if shown, are 4.5" long.
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 23.11 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 314 sqft

+++++
 Nail pattern shown is for PLF loads and point loads converted to PLF loads only. Concentrated loads MUST be distributed to each ply equally. Multi-ply with hangers are based on hanger nails using 1.5" nails min. into the carrying member.
 If shown, use additional fasteners for point loads as indicated from the back plys, distributed symmetrically around the hanger. Use any other approved detail (by others).
 10d = 10d NAILS, SDS = Simpson SDS screws or equivalent substitute
 (*) = Special Connection Req. (by others)
 @ = Indicates load is evenly distributed (no additional fasteners req.)
 + + + + +
 NAIL pattern shown is based on:
 10d BOX = 0.128" dia. x 3.0" long nail
 10d COMMON = 0.148" dia. x 3.0" long nail
 16d BOX = 0.135" dia. x 3.5" long nail
 16d COMMON = 0.162" dia. x 3.5" long nail
 + + + + +

LOAD CASE #1 DESIGN LOADS					
Dir	L.Plf	L.Loc	R.Plf	R.Loc	LL/TL
TC Vert	85.00	0- 0- 0	85.00	23- 6- 0	0.42
BC Vert	15.00	0- 0- 0	15.00	1- 0-12	0.40
BC Vert	7.50	1- 0-12	7.50	23- 0-12	0.40
BC Vert	15.00	23- 0-12	15.00	23- 6- 0	0.40

CRITICAL POINT LOADS			
Type	X.Loc	Max/Dur	Min/Dur
BC Vert	2- 0-12	1150/1.15	-203/1.60
BC Vert	4- 0-12	1150/1.15	-203/1.60
BC Vert	6- 0-12	1150/1.15	-203/1.60
BC Vert	8- 0-12	1150/1.15	-203/1.60
BC Vert	10- 0-12	1287/1.15	-184/1.60
BC Vert	12- 0-12	1397/1.15	-209/1.60
BC Vert	14- 0-12	1397/1.15	-209/1.60
BC Vert	16- 0-12	1411/1.15	-189/1.60
BC Vert	18- 0-12	1411/1.15	-189/1.60
BC Vert	20- 0-12	1411/1.15	-189/1.60
BC Vert	22- 0-12	1411/1.15	-189/1.60



6-2-11 SHIP
 0-4-3
 MAX DEFLECTION (span) :
 L/999 MEM 11-12 (LIVE) LC 10
 L= -0.19" CC= -0.07" CL= -0.26"
 ===== Joint Locations =====

1	0- 0- 0	8	0- 0- 0
2	4- 5-15	9	4- 5-15
3	8- 1- 8	10	8- 1- 8
4	11- 9- 0	11	11- 9- 0
5	15- 4- 8	12	15- 4- 8
6	19- 0- 1	13	19- 0- 1
7	23- 6- 0	14	23- 6- 0

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted. Scale: 3/16" = 1'



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Eng. Job: .EJ. Chk: BJS Dsgnr: BJS	WO: FC BELMONT 2-1-30
TC Live 35.00 psf TC Dead 7.50 psf BC Live 0.00 psf BC Dead 7.50 psf TOTAL 50.00 psf	DurFacs L=1.15 P=1.15 Rep Mbr Bnd 1.15 O.C.Spacing 2- 0- 0 Design Spec IRC-2015 Seqn T6.5.21 - 301457

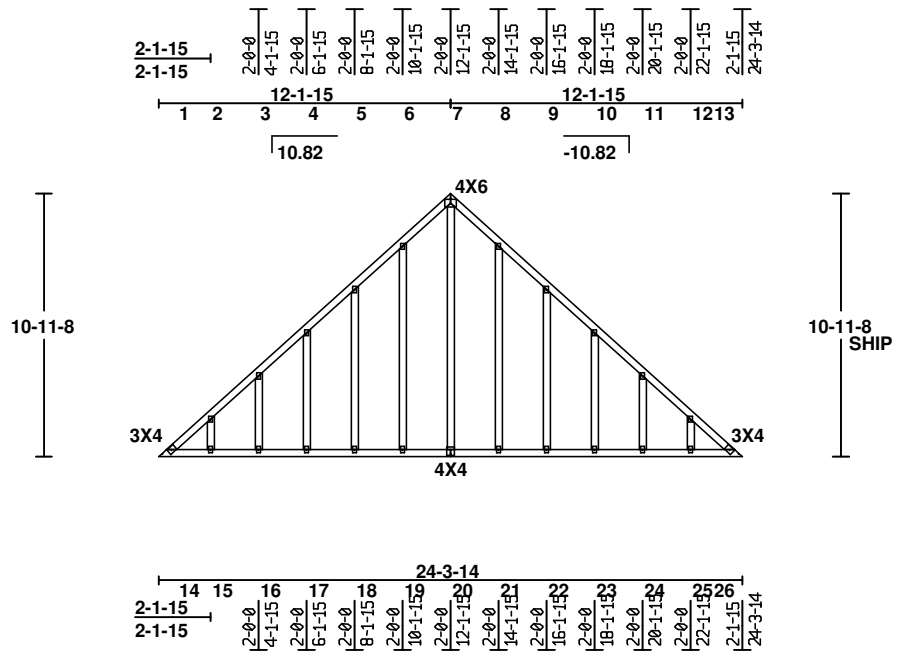
TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 GBL BLK 2x4 DFL STUD
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for
 Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on
 testing and approval as required by IBC 1703
 and ANSI/TPI and are reported in available
 documents as ER-1607 and ESR-1118.
 20 psf Live Load applied to bottom chord in
 accordance with IRC/IBC requirements for
 attics with limited storage having a clear
 height >= 42" and clear width of >= 24".

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the
 lesser of the truss chord lumber value or
 625 for all bearings.
**THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.**
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Gable verticals are 2x 4 web material spaced
 at 24.0 " o.c. unless noted otherwise.
 Top chord supports 24.0 " of uniform load
 at 35 psf live load and 8 psf dead load.
 Additional design considerations may be
 required if sheathing is attached.
 Brace gable studs in accordance with
 Truswal Systems standard gable bracing
 details and charts.
 This truss requires adequate sheathing, as
 designed by others, applied to the truss
 face providing lateral support for webs in
 the truss plane and creating shear wall
 action to resist diaphragm loads.

This truss is designed using the
 ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 25.48 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 49 sqft

==== Joint Locations ====

1	0- 0- 0	14	0- 0- 0
2	2- 1-15	15	2- 1-15
3	4- 1-15	16	4- 1-15
4	6- 1-15	17	6- 1-15
5	8- 1-15	18	8- 1-15
6	10- 1-15	19	10- 1-15
7	12- 1-15	20	12- 1-15
8	14- 1-15	21	14- 1-15
9	16- 1-15	22	16- 1-15
10	18- 1-15	23	18- 1-15
11	20- 1-15	24	20- 1-15
12	22- 1-15	25	22- 1-15
13	24- 3-14	26	24- 3-14



TYPICAL PLATE : 2X3

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

OVER CONTINUOUS SUPPORT

4/15/2019

Scale: 1/8" = 1'



P.O.BOX 787, BLACKFOOT, ID 83221

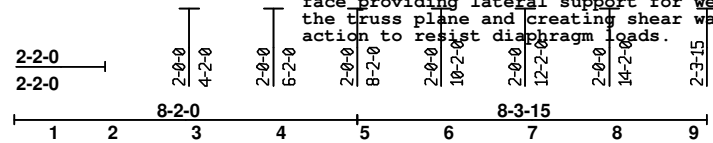
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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.15
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301458

TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 GBL BLK 2x4 DFL STUD
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Drainage must be provided to avoid ponding.
 End verticals are designed for axial loads only unless noted otherwise.
 Extensions above or below the truss profile (if any) have been designed for loads indicated only. Horizontal loads applied at the end of the extensions have not been considered unless shown. A drop-leg to an otherwise unsupported wall may create a hinge effect that requires additional design consideration (by others).

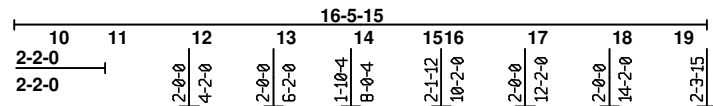
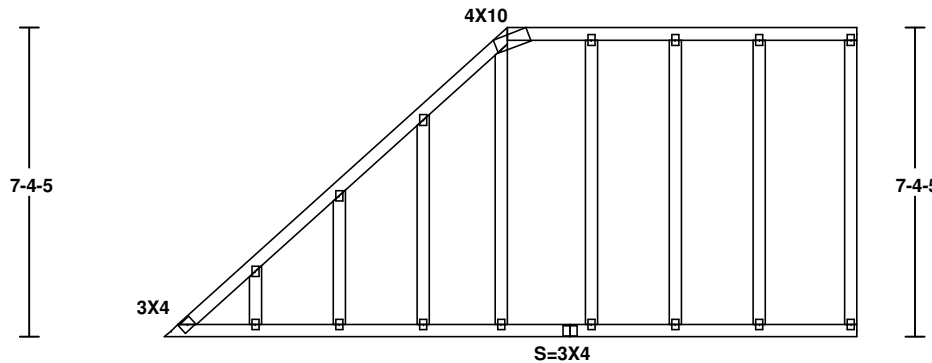
Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Pm = 20 psf
 Gable verticals are 2x 4 web material spaced at 24.0" o.c. unless noted otherwise.
 Top chord supports 24.0" of uniform load at 35 psf live load and 8 psf dead load.
 Additional design considerations may be required if sheathing is attached.
 Brace gable studs in accordance with Truswal Systems standard gable bracing details and charts.
 This truss requires adequate sheathing, as designed by others, applied to the truss face providing lateral support for webs in the truss plane and creating shear wall action to resist diaphragm loads.

This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 23.68 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding
 Tributary Area = 33 sqft
 20 psf bottom chord live load NOT required on this truss, per IRC/IBC requirements for attics with limited storage.



==== Joint Locations ====

1	0-0-0	11	2-2-0
2	2-2-0	12	4-2-0
3	4-2-0	13	6-2-0
4	6-2-0	14	8-0-4
5	8-2-0	15	9-8-0
6	10-2-0	16	10-2-0
7	12-2-0	17	12-2-0
8	14-2-0	18	14-2-0
9	16-5-15	19	16-5-15
10	0-0-0		



TYPICAL PLATE : 2X3

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

4/15/2019

OVER CONTINUOUS SUPPORT

Scale: 7/32" = 1'

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Eng. Job: .EJ.
 Chk: BJS
 Dsgnr: BJS

TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO: FC BELMONT 2-1-30
DurFacs L=1.15 P=1.15
Rep Mbr Bnd 1.15
O.C.Spacing 2-0-0
Design Spec IRC-2015
Seqn T6.5.21 - 301459

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	419	5.50"	1.50"
2	3-11- 4	150	1.50"	1.50"
3	3-11- 4	64	1.50"	1.50"

TC	FORCE	AXL	BND	CSI
1-2	-98	0.00	0.19	0.19
2-0	-3	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
3-4	142	0.01	0.09	0.10
4-0	0	0.00	0.00	0.00

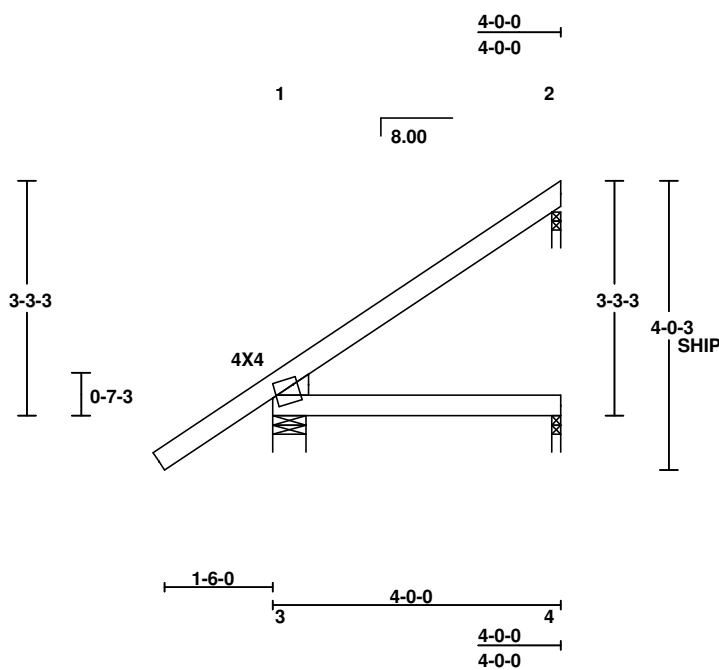
TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 WEDGE 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Install interior support(s) before erection.
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Mark all interior bearing locations.
 Shim bearings (if needed) for req. support.

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -99 lb
 2 -134 lb
 3 -6 lb
 HORIZONTAL REACTION(S) :
 support 1 142 lb
 support 3 142 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 21.46 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 8 sqft

MAX DEFLECTION (span) :
 L/999 MEM 1-2 (LIVE) LC 1
 L= -0.03" CC= -0.01" CL= -0.04"

==== Joint Locations =====
 1 0- 0- 0 3 0- 0- 0
 2 4- 0- 0 4 4- 0- 0



OVER 3 SUPPORTS

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 3/8" = 1'



P.O. BOX 787, BLACKFOOT, ID 83221

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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO: FC BELMONT 2-1-30
DurFacs L=1.15 P=1.15
Rep Mbr Bnd 1.15
O.C.Spacing 2- 0- 0
Design Spec IRC-2015
Seqn T6.5.21 - 301460

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	519	5.50"	1.50"
2	5-11- 4	227	1.50"	1.50"
3	5-11- 4	98	1.50"	1.50"

TC	FORCE	AXL	BND	CSI
1-2	-145	0.00	0.48	0.48
2-0	-3	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
3-4	215	0.00	0.25	0.25
4-0	0	0.00	0.00	0.00

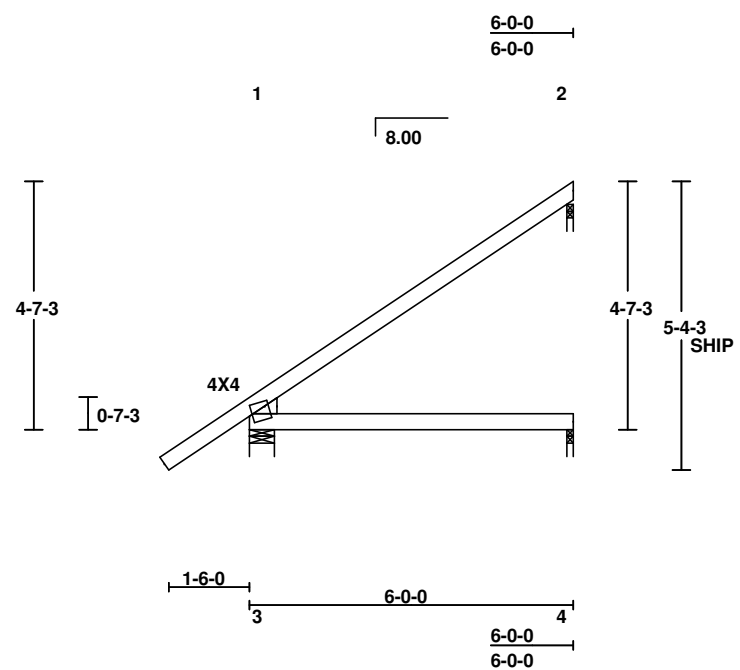
TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 WEDGE 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for
 Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on
 testing and approval as required by IBC 1703
 and ANSI/TPI and are reported in available
 documents as ER-1607 and ESR-1118.
 Install interior support(s) before erection.
 20 psf bottom chord live load NOT required
 on this truss, per IBC/IRC requirements for
 attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the
 lesser of the truss chord lumber value or
 625 for all bearings.
**THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.**
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Mark all interior bearing locations.
 Shim bearings (if needed) for req. support.

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -113 lb
 2 -194 lb
 3 -5 lb
 HORIZONTAL REACTION(S) :
 support 1 214 lb
 support 3 214 lb
 This truss is designed using the
 ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.12 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 12 sqft

MAX DEFLECTION (span) :
 L/437 MEM 1-2 (LIVE) LC 1
 L= -0.16" CC= -0.04" CL= -0.20"

==== Joint Locations =====
 1 0- 0- 0 3 0- 0- 0
 2 6- 0- 0 4 6- 0- 0



OVER 3 SUPPORTS

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted. Scale: 9/32" = 1'



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Eng. Job: .EJ. Chk: BJS Dsgnr: BJS	WO: FC BELMONT 2-1-30
TC Live 35.00 psf TC Dead 7.50 psf BC Live 0.00 psf BC Dead 7.50 psf TOTAL 50.00 psf	DurFacs L=1.15 P=1.15 Rep Mbr Bnd 1.15 O.C.Spacing 2- 0- 0 Design Spec IRC-2015 Seqn T6.5.21 - 301461

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	519	5.50"	1.50"
2	5-11- 4	162	1.50"	1.50"
3	5-11- 4	130	1.50"	1.50"

TC	FORCE	AXL	BND	CSI
1-2	-306	0.00	0.06	0.06
2-3	-92	0.00	0.22	0.22
3-0	-3	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
4-5	203	0.03	0.03	0.06
5-6	194	0.02	0.19	0.21
6-7	215	0.00	0.21	0.21
7-0	0	0.00	0.00	0.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	97	0.03	2-6	243	0.06

TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 WEB 2x4 DFL STUD
 WEDGE 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Install interior support(s) before erection. 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

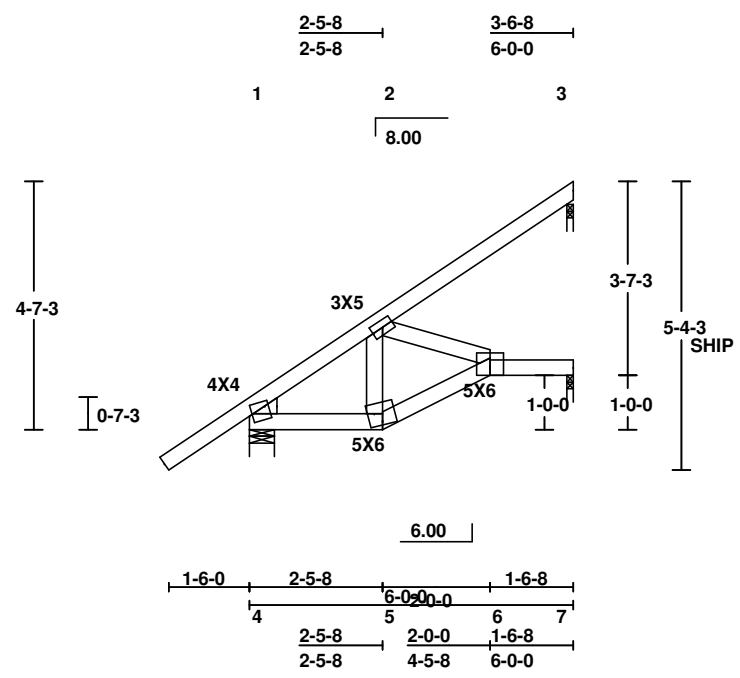
[PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Mark all interior bearing locations.
 Shim bearings (if needed) for req. support.

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -148 lb
 2 -125 lb
 3 -75 lb
 HORIZONTAL REACTION(S) :
 support 1 214 lb
 support 3 214 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.12 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 12 sqft

MAX DEFLECTION (span) :
 L/909 MEM 5-6 (LIVE) LC 1
 L= -0.08" CC= -0.04" CL= -0.12"

==== Joint Locations =====

1	0- 0- 0	5	2- 5- 8
2	2- 5- 8	6	4- 5- 8
3	6- 0- 0	7	6- 0- 0
4	0- 0- 0		



OVER 3 SUPPORTS

4/15/2019

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Scale: 9/32" = 1'



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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.15
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301462

BRG	X-LOC	REACT	SIZE	REQ'D	TC	2x4	DFL	#1 & Btr.
1	0- 2-12	440	5.50"	1.50"	BC	2x4	DFL	#1 & Btr.
2	4- 4- 5	157	1.50"	1.50"	WEB	2x4	DFL	STUD
3	4- 4- 5	55	1.50"	1.50"	WEDGE	2x4	DFL	#1 & Btr.

TC	FORCE	AXL	BND	CSI
1-2	-128	0.00	0.21	0.21
2-3	-96	0.00	0.22	0.22
3-0	-3	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
4-5	158	0.01	0.11	0.12
5-6	164	0.01	0.10	0.12
6-0	-1	0.00	0.00	0.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	60	0.01			

Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance using IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Install interior support(s) before erection.
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

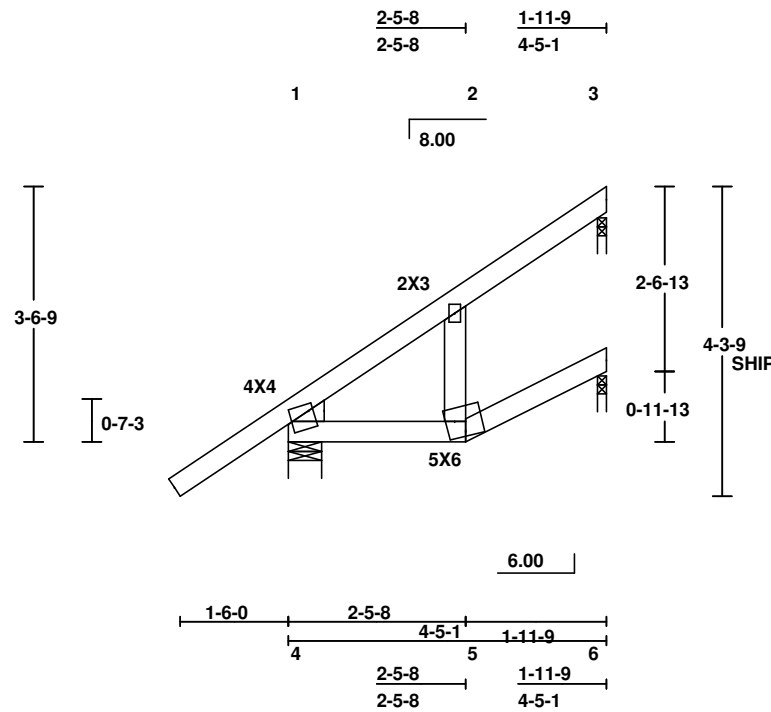
[PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Mark all interior bearing locations.
 Shim bearings (if needed) for req. support.

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -138 lb
 2 -154 lb
 3 -24 lb
 HORIZONTAL REACTION(S) :
 support 1 158 lb
 support 3 158 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 21.60 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 9 sqft

MAX DEFLECTION (span) :
 L/647 MEM 4-5 (LIVE) LC 22
 L= 0.08" CC= -0.01" CL= 0.07"

==== Joint Locations =====

1	0- 0- 0	4	0- 0- 0
2	2- 5- 8	5	2- 5- 8
3	4- 5- 1	6	4- 5- 1



OVER 3 SUPPORTS

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 3/8" = 1'



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Eng. Job:	.EJ.	WO:	FC BELMONT 2-1-30
Chk:	BJS		
Dsgnr:	BJS		
TC Live	35.00 psf	DurFacs	L=1.15 P=1.15
TC Dead	7.50 psf	Rep Mbr Bnd	1.15
BC Live	0.00 psf	O.C.Spacing	2- 0- 0
BC Dead	7.50 psf	Design Spec	IRC-2015
TOTAL	50.00 psf	Seqn	T6.5.21 - 301463

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	365	5.50"	1.50"
2	2-10- 5	108	1.50"	1.50"
3	2-10- 5	46	1.50"	1.50"

TC	FORCE	AXL	BND	CSI
1-2	-71	0.00	0.09	0.09
2-0	-3	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
3-4	101	0.01	0.05	0.06
4-0	0	0.00	0.00	0.00

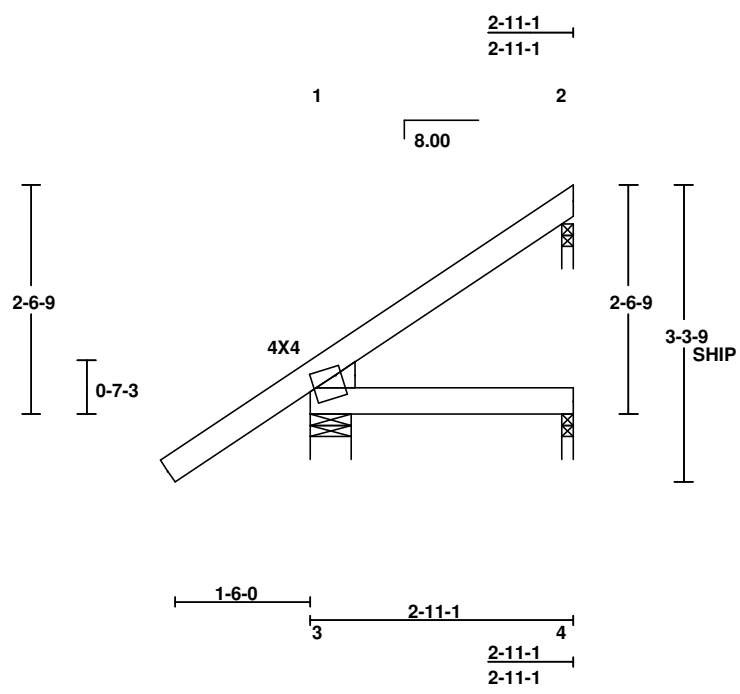
TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 WEDGE 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Install interior support(s) before erection.
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Mark all interior bearing locations.
 Shim bearings (if needed) for req. support.

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -89 lb
 2 -98 lb
 3 -7 lb
 HORIZONTAL REACTION(S) :
 support 1 101 lb
 support 3 101 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 21.10 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 6 sqft

MAX DEFLECTION (span) :
 L/999 MEM 1-2 (LIVE) LC 1
 L= -0.01" CC= 0.00" CL= -0.01"

==== Joint Locations =====
 1 0- 0- 0 3 0- 0- 0
 2 2-11- 1 4 2-11- 1



OVER 3 SUPPORTS

4/15/2019

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Scale: 1/32" = 1'



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Eng. Job: .EJ. Chk: BJS Dsgnr: BJS	WO: FC BELMONT 2-1-30
TC Live 35.00 psf TC Dead 7.50 psf BC Live 0.00 psf BC Dead 7.50 psf TOTAL 50.00 psf	DurFacs L=1.15 P=1.15 Rep Mbr Bnd 1.15 O.C.Spacing 2- 0- 0 Design Spec IRC-2015 Seqn T6.5.21 - 301464

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	292	5.50"	1.50"
2	1- 4- 5	46	1.50"	1.50"
3	1- 4- 5	21	1.50"	1.50"

TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 WEDGE 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Install interior support(s) before erection.
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Mark all interior bearing locations.
 Shim bearings (if needed) for req. support.

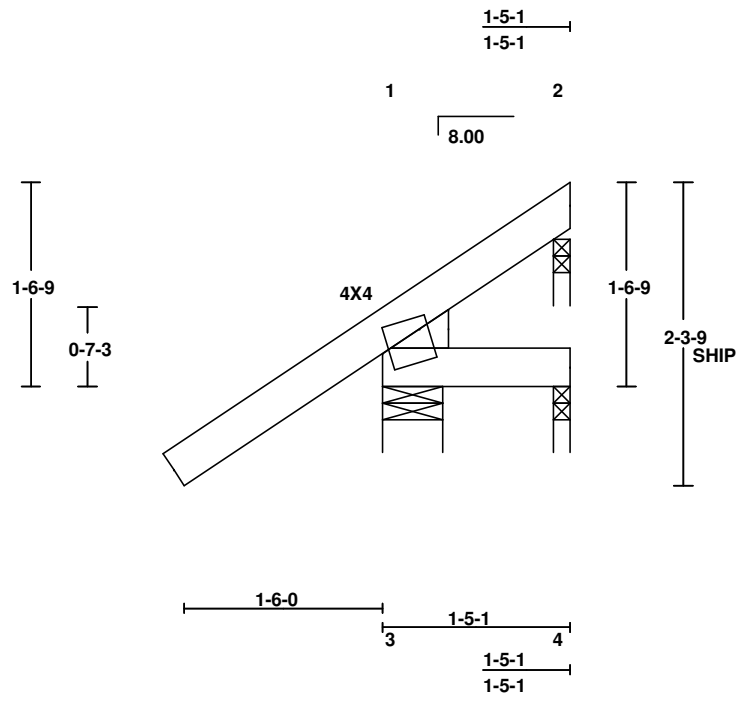
UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -78 lb
 2 -48 lb
 3 -9 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 20.60 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding
 Tributary Area = 3 sqft

TC	FORCE	AXL	BND	CSI
1-2	-33	0.00	0.01	0.01
2-0	-3	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
3-4	45	0.00	0.01	0.02
4-0	0	0.00	0.00	0.00

MAX DEFLECTION (span) :
 L/999 MEM 3-4 (LIVE) LC 19
 L= 0.00" CC= 0.00" CL= 0.00"

==== Joint Locations =====
 1 0- 0- 0 3 0- 0- 0
 2 1- 5- 1 4 1- 5- 1



OVER 3 SUPPORTS

4/15/2019

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Scale: 11/16" = 1'



P.O. BOX 787, BLACKFOOT, ID 83221

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Eng. Job: .EJ.	WO: FC BELMONT 2-1-30
Chk: BJS	
Dsgnr: BJS	
TC Live 35.00 psf	DurFacs L=1.15 P=1.15
TC Dead 7.50 psf	Rep Mbr Bnd 1.15
BC Live 0.00 psf	O.C.Spacing 2- 0- 0
BC Dead 7.50 psf	Design Spec IRC-2015
TOTAL 50.00 psf	Seqn T6.5.21 - 301465

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 2-12	373	5.50"	1.50"
2	3- 0- 1	110	1.50"	1.50"
3	3- 0- 1	47	1.50"	1.50"

TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Mark all interior bearing locations. Shim bearings (if needed) for req. support. 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Install interior support(s) before erection.

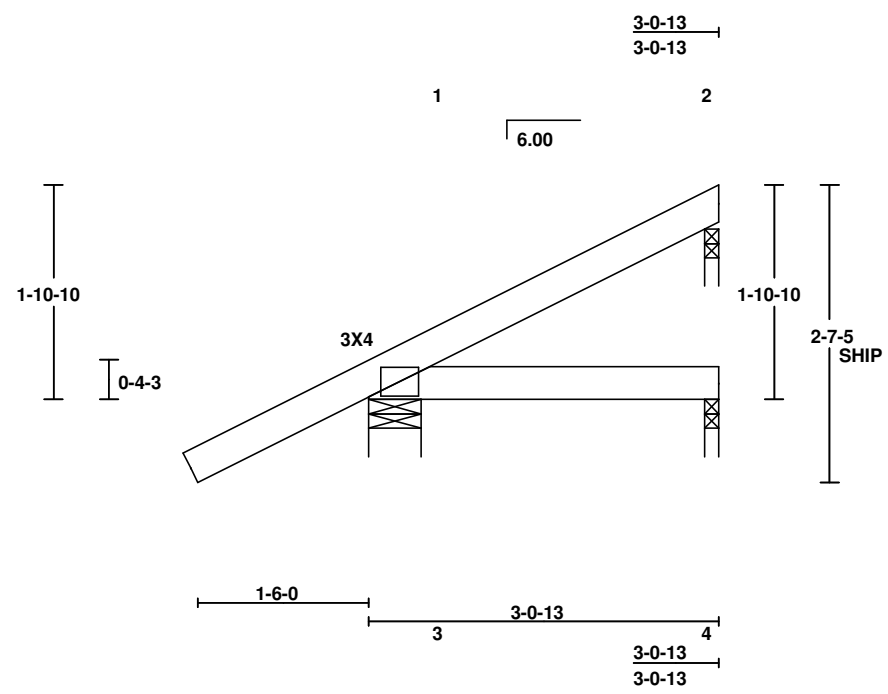
UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -128 lb
 2 -77 lb
 3 -2 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 20.78 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding
 Tributary Area = 6 sqft

TC	FORCE	AXL	BND	CSI
1-2	-49	0.00	0.09	0.09
2-0	-2	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
3-4	77	0.00	0.06	0.06
4-0	0	0.00	0.00	0.00

MAX DEFLECTION (span) :
 L/999 MEM 1-2 (LIVE) LC 47
 L= -0.01" CC= 0.00" CL= -0.01"

==== Joint Locations =====
 1 0- 0- 0 3 0- 0- 0
 2 3- 0-13 4 3- 0-13



OVER 3 SUPPORTS

4/15/2019

All plates are 20 gauge Truswal Connectors unless preceded by "MX" for HS 20 gauge or "H" for 16 gauge, positioned per Joint Detail Reports available from Truswal software, unless noted.

Scale: 19/32" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301466

BRG	X-LOC	REACT	SIZE	REQ'D
1	0- 0-12	274	1.50"	1.50"
2	5- 5- 1	156	1.50"	1.50"
3	5- 5- 1	118	1.50"	1.50"

TC	FORCE	AXL	BND	CSI
1-2	-340	0.00	0.09	0.09
2-3	67	0.00	0.20	0.20
3-0	-2	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
4-5	255	0.03	0.02	0.06
5-6	247	0.03	0.16	0.20
6-7	146	0.00	0.20	0.20
7-0	0	0.00	0.00	0.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	89	0.03	2-6	-259	0.05

TC 2x4 DFL #1 & Btr.
 BC 2x4 DFL #1 & Btr.
 WEB 2x4 DFL STUD
 Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Mark all interior bearing locations.
 Shim bearings (if needed) for req. support.
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

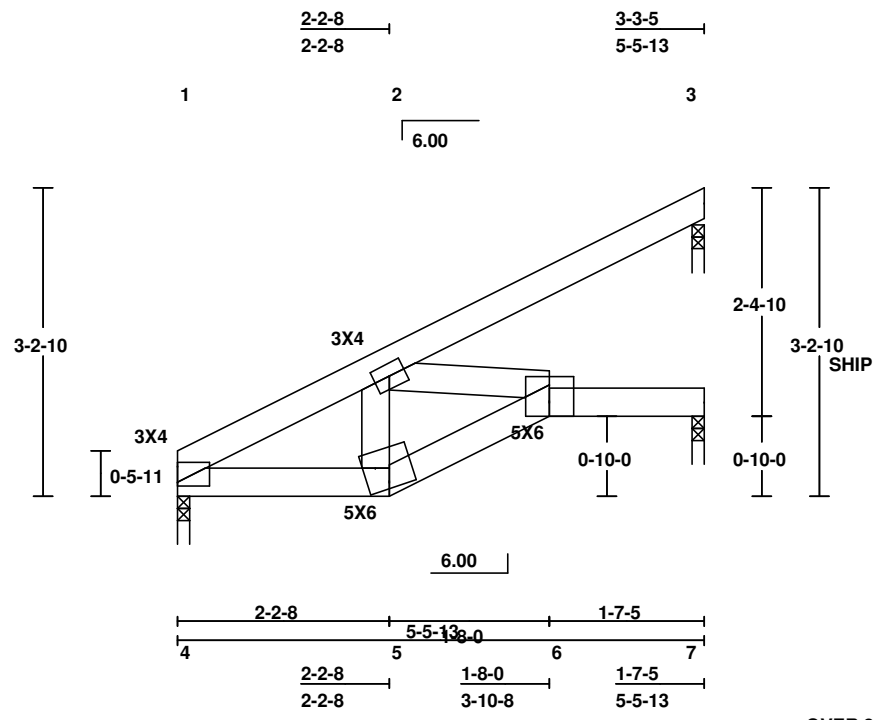
[PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Install interior support(s) before erection.

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -85 lb
 2 -102 lb
 3 -47 lb
 HORIZONTAL REACTION(S) :
 support 1 146 lb
 support 3 146 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 21.45 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 11 sqft

MAX DEFLECTION (span) :
 L/966 MEM 5-6 (LIVE) LC 47
 L= -0.07" CC= -0.04" CL= -0.10"

==== Joint Locations =====

1	0- 0- 0	5	2- 2- 8
2	2- 2- 8	6	3-10- 8
3	5- 5-13	7	5- 5-13
4	0- 0- 0		



OVER 3 SUPPORTS

4/15/2019

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Scale: 1/2" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.15
O.C.Spacing	2- 0- 0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301467

BRG	X-LOC	REACT	SIZE	REQ'D	TC
1	0-4-2	1007	8.19"	1.50"	2x4 DFL #1 & Btr.
2	9-10-0	46	1.50"	1.50"	BC 2x4 DFL #1 & Btr.
3	9-10-0	723	1.50"	1.50"	WEB 2x4 DFL STUD
					WEDGE 2x4 DFL #1 & Btr.

TC	FORCE	AXL	BND	CSI
1-2	-1025	0.01	0.28	0.29
2-3	-873	0.01	0.21	0.22
3-4	-43	0.00	0.17	0.17
4-0	-2	0.00	0.00	0.00

BC	FORCE	AXL	BND	CSI
5-6	893	0.12	0.31	0.43
6-7	975	0.12	0.11	0.23
7-8	693	0.09	0.02	0.11
8-0	0	0.00	0.00	0.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-6	-237	0.05	3-7	504	0.17
2-7	-157	0.05	3-8	-990	0.24

Kcr (creep factor) = 2.00
 Refer to Joint QC Detail Sheet for Maximum Rotational Tolerance used
 IRC/IBC truss plate values are based on testing and approval as required by IBC 1703 and ANSI/TPI and are reported in available documents as ER-1607 and ESR-1118.
 Install interior support(s) before erection. Shim bearings (if needed) for req. support.
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

[PM]=PLATE MONITOR USED-See Joint Report
 Designed per ANSI/TPI 1-2014
 Fabrication Tolerance = 20.0%
 Bearings designed for an FcPerp value of the lesser of the truss chord lumber value or 625 for all bearings.
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 ASCE7-10 SNOW LOAD DESIGN CRITERIA:
 Pg = 50 psf, Ce = 1.0, I = 1.0, Ct = 1.00
 Mark all interior bearing locations.
 Permanent bracing is required (by others) to prevent rotation/toppling. See BCSI and ANSI/TPI 1.

UPLIFT REACTION(S) :
 Support C&C Wind Non-Wind
 1 -428 lb
 2 -10 lb
 3 -367 lb
 HORIZONTAL REACTION(S) :
 support 3 111 lb
 This truss is designed using the ASCE7-10 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = Not End Zone
 Exp Category = C
 Bldg Length = 99.00 ft, Bldg Width = 50.00 ft
 Mean roof height = 22.11 ft, mph = 115
 Occupancy Category II, Dead Load = 9.0 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 34 sqft

-----LOAD CASE #1 DESIGN LOADS-----

Dir	L.Plf	L.Loc	R.Plf	R.Loc	LL/TL
TC Vert	100.00	2-6-0	100.00	0-0-0	0.70
TC Vert	85.00	0-0-0	85.00	1-5-15	0.42
TC Vert	42.50	1-5-15	42.50	8-5-15	0.42
TC Vert	85.00	8-5-15	85.00	9-10-12	0.42
BC Vert	15.00	0-0-0	15.00	1-5-15	0.00
BC Vert	7.50	1-5-15	7.50	8-5-15	0.00
BC Vert	15.00	8-5-15	15.00	9-10-12	0.00

...Type... lbs X.Loc LL/TL
 TC Vert 60.0 - 2-6-0 1.00

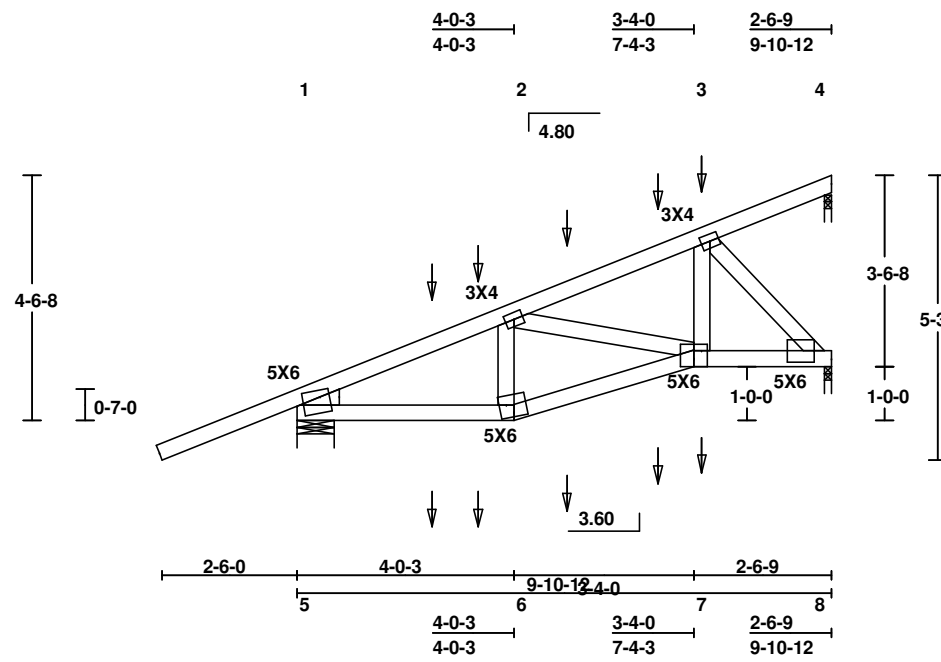
-----CRITICAL POINT LOADS-----

Type	X.Loc	Max/Dur	Min/Dur
TC Vert	2-5-15	46/1.15	-48/1.60
TC Vert	3-4-4	109/1.15	-77/1.60
TC Vert	4-11-15	107/1.15	-98/1.60
TC Vert	6-8-4	156/1.15	-102/1.60
TC Vert	7-5-15	156/1.15	-154/1.60
BC Vert	2-5-15	20/1.25	-9/1.60
BC Vert	3-4-4	47/1.25	-2/1.60
BC Vert	4-11-15	46/1.25	-7/1.60
BC Vert	6-8-4	118/1.15	-47/1.60
BC Vert	7-5-15	55/1.15	-24/1.60

MAX DEFLECTION (span) :
 L/999 MEM 2-3 (LIVE) LC 1
 L = -0.05" CC = -0.02" CL = -0.07"

===== Joint Locations =====

1	0-0-0	5	0-0-0
2	4-0-3	6	4-0-3
3	7-4-3	7	7-4-3
4	9-10-12	8	9-10-12



OVER 3 SUPPORTS

4/15/2019

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Scale: 9/32" = 1'



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Eng. Job:	.EJ.
Chk:	BJS
Dsgnr:	BJS
TC Live	35.00 psf
TC Dead	7.50 psf
BC Live	0.00 psf
BC Dead	7.50 psf
TOTAL	50.00 psf

WO:	FC BELMONT 2-1-30
DurFacs	L=1.15 P=1.15
Rep Mbr Bnd	1.00
O.C.Spacing	2-0-0
Design Spec	IRC-2015
Seqn	T6.5.21 - 301468